
**CONCEPTUAL HYDROLOGY STUDY
FOR
THE DIAMOND**

**In the City of
LAKE ELSINORE, CALIFORNIA**

Prepared For:

JIC-CP Diamond Development, LLC

Prepared By:

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- EXISTING HYDROLOGY MAP SHEET 1 OF 1
- EXISTING DETENTION BASIN MAP SHEET 1 OF 1
- PROPOSED HYDROLOGY MAP SHEET 1 OF 1

I. PROJECT DESCRIPTION

The Diamond Project is a development of approximately 87.2 acres, consisting of approximately 44.2 acres of Mixed Use including the existing Diamond Stadium, 23.7 acres of Commercial Development, 2.0 acres of Open Space, 5.6 acres of Roads, and 11.7 acres of Residential Development.. The project site may be viewed as two distinct segments. The portion north of Malaga Road around the existing Diamond Stadium is commercial development. The south portion of the project is residential development. A portion of the project is located within the FEMA Special Flood Hazards Zone AE (100-yr frequency base flood elevation of 1263). The southwesterly open space area is proposed for use in providing compensating flood storage for the project in conformance with conditions contained in the ACOE Lake Management Project 404 permit.

II. LEVEL OF PROTECTION

The Riverside County Subdivision Ordinance (Number 460) has required protection of all new subdivisions from the 100-year flood event. This criteria has been the basis for the preliminary design of all street grades and storm drain improvements within the project.

For the purpose of the storm drain design, all sizing will be based on the 100 year storm. To protect properties from flooding when the water surface reaches 1263.3, secondary overflows have been located at all sump locations throughout the development to ensure flood waters are capable of flowing on the surface to the designated flood storage areas. Secondary overflows will also provide protection in the event of clogging of the catch basins during the smaller storms.

The project is not located within a County of Riverside drainage area master plan.

In accordance with the City of Lake Elsinore municipal code, all finished floor elevations within the proposed development must be set at elevation 1267.0, which is 3.7 feet above the 100-year flood water surface elevation of 1263.3.

III. HYDROLOGY

The hydrologic calculations were performed in accordance with the County of Riverside Hydrology Manual (April 1978). "CIVILDESIGN" software, marketed by Joseph Bonadiman & Associates, Inc. of San Bernardino, was used to perform the drainage models of the site.

Rainfall

The computer program stores a rainfall database corresponding to the County of Riverside Hydrology Manual (October 1986), which provides the required time versus intensity data for various areas within Riverside County.

Infiltration

Based on the soil maps in the Hydrology Manual, soil type "D" was used in the hydrology calculations. The computer program calculates run-off loss rates based on the selected soil type and various development types.

Rational Method

The Rational Method was used to determine peak flow rates. This information will be used for determining the location and sizes of the proposed storm drain systems.

Existing Drainage

As shown in the Existing Hydrology Exhibit, the Diamond Project currently drains to five outlet concentration points. Sub-area A collects the majority of the site (58.2 acres) including the Stadium, several vacant lots, and the roadway drainage accumulated along Pete Lehr Drive, Railroad Canyon Road, Campbell Street, and a portion of Lakeshore/Mission Trail Drive. This flow is collected and discharged at the terminus of Pete Lehr Drive and empties directly into Lake Elsinore. Sub-area B conveys a portion of the existing Stadium parking lot drainage and also discharges directly into Lake Elsinore through a separate drainage channel. Sub-area C collects drainage from the remainder of the Stadium parking lot, Malaga Road, and a portion of the temporary Stadium overflow parking lot. This flow is collected and discharged to water quality features and a detention basin with the Summerly Development adjacent to the golf course. Storm water within the detention basin is discharged into the remainder of the back basin of Lake Elsinore by pumping. Sub-Area D and E collect the existing development at the north end of the project and discharge directly into the Lake Elsinore inlet channel through existing storm drain connections. The 100-year discharge associated with these five existing sub-areas is 205.7 cfs.

Developed Condition

As shown in the Proposed Hydrology Exhibit, the proposed Diamond Project will drain to three outlet concentration points. Sub-area A will collect the majority of the project (69.1 acres) through the proposed backbone storm drain system within the proposed roadway (Diamond) as well as the existing roadway drainage accumulated along Railroad Canyon Road, Campbell Street, and a portion of Lakeshore/Mission Trail Drive. This backbone storm drain system will parallel the existing storm drain system located at the end of Malaga Road and will discharge to water quality features and a detention basin with the Summerly Development adjacent to the golf course. Sub-area B will utilize the existing

drainage outlet at the north end of the project. Water quality for this sub-area will utilize grassy drainage swales as the flow discharges directly into the Lake Elsinore inlet channel. Sub-Area C collects the existing drainage from the majority of the Stadium parking lot, Malaga Road, and the proposed development south of Malaga. The drainage patterns for this area remain unchanged and will continue to discharge to the existing storm drain located at the end of Malaga. The 100-year discharge associated with these three proposed sub-areas is 226.7 cfs. The existing detention area within the Summerly Development is proposed to serve the increased storm flows.

Appendix C presents an analysis of the proposed project storm flows to the existing detention basin with the Summerly Development. The analysis presents the storm drain facilities required for conveyance of project flows to the existing detention basin. Expected water surface elevations within the detention basin based on project storm flows are presented. The increase in water surface elevations due to the project are insignificant as compared to the existing condition and do not have an adverse affect on the Summerly golf course.

Flood Storage Volumes

The existing flood storage volume within the project area is estimated at 39 acre-feet (up to elevation 1263.3 NAVD 29). The proposed plan provides for 22 acre-feet of flood storage with the additional 17 acre-feet of flood storage being provided within the open space to the southwest of the project.

IV. CONCLUSION

The proposed Diamond Drainage Plan provides for safe conveyance of storm water from the project site to existing drainage infrastructure constructed with the Summerly Development on the south side of the project. The proposed improvements yield an approximate 10 percent increase in peak discharge values. The Drainage Plan utilizes previously planned storm water detention, water quality features and flood control measures within the back basin of Lake Elsinore. New detention facilities are therefore not required for the project. The drainage facilities within the project are to be sized to convey the 100-year frequency peak storm flows. Additional storm drain facilities within the Summerly detention and water quality pond areas are required to convey the project storm flows. The drainage facilities are to be designed in conformance with City of Lake Elsinore and Riverside County Flood Control and Water Conservation District standards.

V. REFERENCES

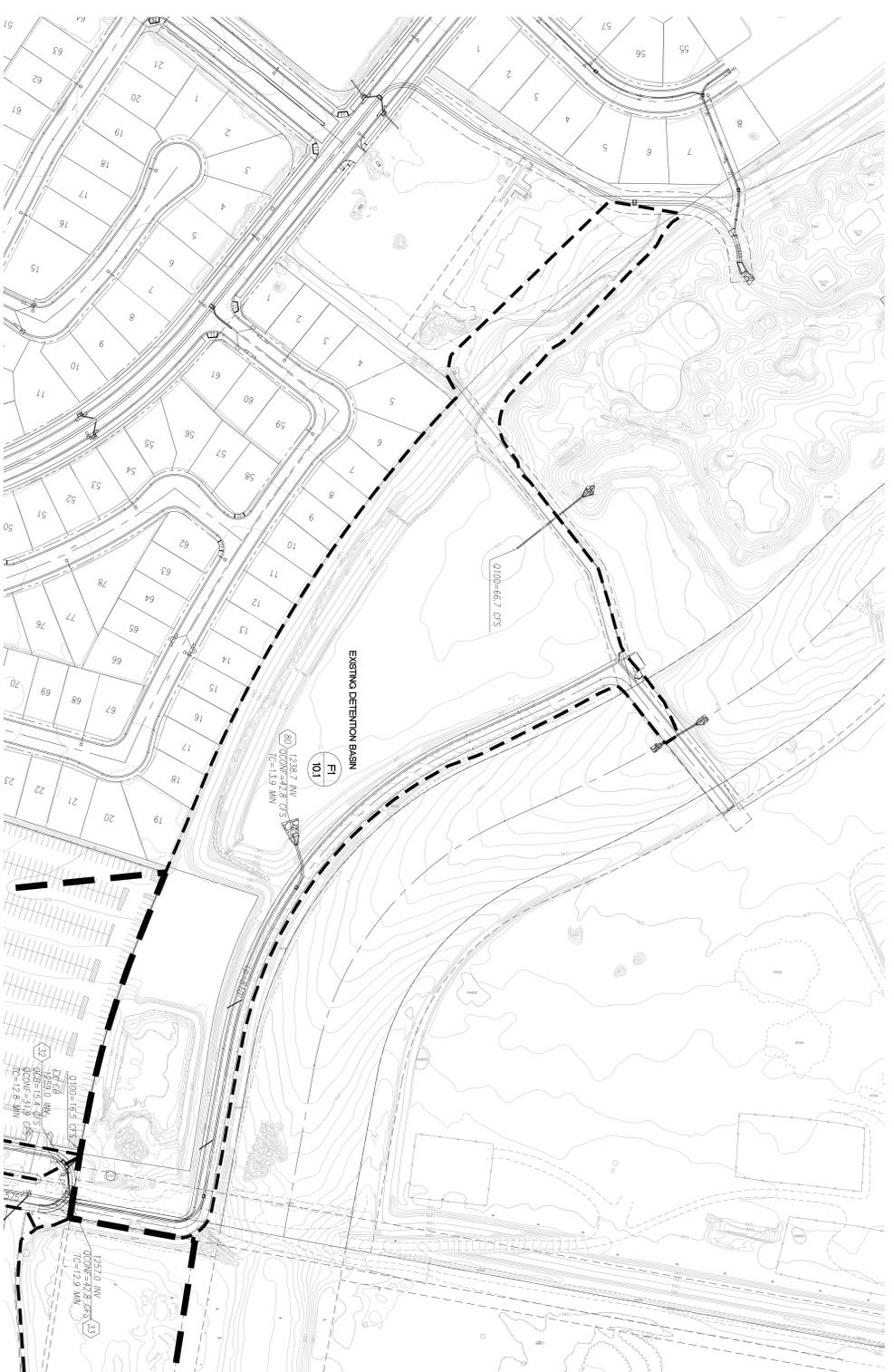
1. Hydrology Manual, Riverside County Flood Control and Water Conservation District, April 1978.
2. CIVILDESIGN, Bonadiman and Associates, 1998.



PROJECT LOCATION MAP

FIGURE 1

APPENDIX A: EXISTING 100-YEAR HYDROLOGY CALCULATIONS

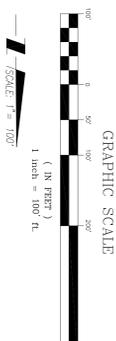


SUMMARY TABLE

SUB-AREA F	0100 (CFS) AT OUTLET	TRIBUTARY AREA (ACRES)
	66.7 CFS	24.9 AC



- 010 10-YEAR PEAK FLOW IN CFS
- 0100 100-YEAR PEAK FLOW IN CFS
- 03 3% TIME OF CONCENTRATION IN MIN
- 03 3% TIME OF CONCENTRATION (100-YEAR STORM) IN MIN
- MANIFOLD
- MANIFOLD ELEVATION
- EXISTING GRADE ELEVATION
- FINISH GRADE ELEVATION
- FINISH SURFACE ELEVATION
- LENGTH OF FLOWPATH IN FEET



NO.	DATE	REVISIONS

DESIGNED BY:
 DRAFTED BY:
 CHECKED BY:
 DATE:

PROJECT NO. 1008800
 SHEET 2 OF 2

DIAMOND CENTER
 EXISTING HYDROLOGY

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 RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM BASED ON
 RIVERSIDE COUNTY FLOOD CONTROL & WATER CONSERVATION DISTRICT
 (RCFC&WCD) 1978 HYDROLOGY MANUAL
 (c) Copyright 1982-2007 Advanced Engineering Software (aes)
 (Rational Tabling Version 7.0D)
 Release Date: 06/01/2007 License ID 1557

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
 * EXISTING HYDROLOGY ANALYSIS FOR DIAMOND CENTER *
 * 100-YEAR RUN *
 * BY KAM 030409 *

FILE NAME: DIAM-EX.DAT
 TIME/DATE OF STUDY: 09:54 03/11/2009

 USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

USER SPECIFIED STORM EVENT(YEAR) = 100.00
 SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
 SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
 10-YEAR STORM 10-MINUTE INTENSITY(INCH/HOUR) = 2.320
 10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.980
 100-YEAR STORM 10-MINUTE INTENSITY(INCH/HOUR) = 3.540
 100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.500
 SLOPE OF 10-YEAR INTENSITY-DURATION CURVE = 0.4809628
 SLOPE OF 100-YEAR INTENSITY-DURATION CURVE = 0.4792280
 COMPUTED RAINFALL INTENSITY DATA:

STORM EVENT = 100.00 1-HOUR INTENSITY(INCH/HOUR) = 1.500
 SLOPE OF INTENSITY DURATION CURVE = 0.4792

RCFC&WCD HYDROLOGY MANUAL "C"--VALUES USED FOR RATIONAL METHOD
 NOTE: COMPUTE CONFLUENCE VALUES ACCORDING TO RCFC&WCD HYDROLOGY MANUAL
 AND IGNORE OTHER CONFLUENCE COMBINATIONS FOR DOWNSTREAM ANALYSES

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/ SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 21 A1

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL

TC = $K * [(LENGTH**3) / (ELEVATION CHANGE)]**0.2$
INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
UPSTREAM ELEVATION(FEET) = 1276.00
DOWNSTREAM ELEVATION(FEET) = 1273.50
ELEVATION DIFFERENCE(FEET) = 2.50
TC = $0.303 * [(300.00**3) / (2.50)]**0.2 = 7.732$
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.004
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8910
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 1.43
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 1.43

FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 51 A2

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1273.50 DOWNSTREAM(FEET) = 1264.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 540.00 CHANNEL SLOPE = 0.0176
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 99.990
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.058
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7858
SOIL CLASSIFICATION IS "D"
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 8.27
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.54
AVERAGE FLOW DEPTH(FEET) = 0.23 TRAVEL TIME(MIN.) = 5.84
Tc(MIN.) = 13.58
SUBAREA AREA(ACRES) = 5.60 SUBAREA RUNOFF(CFS) = 13.45
TOTAL AREA(ACRES) = 6.0 PEAK FLOW RATE(CFS) = 14.88

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.29 FLOW VELOCITY(FEET/SEC.) = 1.81
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 12.00 = 840.00 FEET.

FLOW PROCESS FROM NODE 12.00 TO NODE 13.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1259.90 DOWNSTREAM(FEET) = 1258.00
FLOW LENGTH(FEET) = 309.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 17.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.19
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 14.88
PIPE TRAVEL TIME(MIN.) = 0.83 Tc(MIN.) = 14.41
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 1149.00 FEET.

```

*****
FLOW PROCESS FROM NODE      13.00 TO NODE      13.00 IS CODE = 81  A3
-----
>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.972
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8883
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 7.70 SUBAREA RUNOFF(CFS) = 20.33
TOTAL AREA(ACRES) = 13.7 TOTAL RUNOFF(CFS) = 35.21
TC(MIN.) = 14.41
*****
FLOW PROCESS FROM NODE      13.00 TO NODE      14.00 IS CODE = 31
-----
>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1258.00 DOWNSTREAM(FEET) = 1256.10
FLOW LENGTH(FEET) = 550.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 27.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.13
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 35.21
PIPE TRAVEL TIME(MIN.) = 1.49 Tc(MIN.) = 15.90
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 14.00 = 1699.00 FEET.
*****
FLOW PROCESS FROM NODE      14.00 TO NODE      14.00 IS CODE = 1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 15.90
RAINFALL INTENSITY(INCH/HR) = 2.83
TOTAL STREAM AREA(ACRES) = 13.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 35.21
*****
FLOW PROCESS FROM NODE      15.00 TO NODE      16.00 IS CODE = 21  A4
-----
>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 250.00
UPSTREAM ELEVATION(FEET) = 1284.40
DOWNSTREAM ELEVATION(FEET) = 1283.00
ELEVATION DIFFERENCE(FEET) = 1.40
TC = 0.303*[(250.00**3)/(1.40)]**.2 = 7.783
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.992
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8910
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 2.13
TOTAL AREA(ACRES) = 0.60 TOTAL RUNOFF(CFS) = 2.13

```

FLOW PROCESS FROM NODE 16.00 TO NODE 17.00 IS CODE = 61 A5

>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1283.00 DOWNSTREAM ELEVATION(FEET) = 1269.30
STREET LENGTH(FEET) = 1207.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 20.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 4.47
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.40
HALFSTREET FLOOD WIDTH(FEET) = 12.02
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.74
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.09
STREET FLOW TRAVEL TIME(MIN.) = 7.35 Tc(MIN.) = 15.13
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.903
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8880
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.80 SUBAREA RUNOFF(CFS) = 4.64
TOTAL AREA(ACRES) = 2.4 PEAK FLOW RATE(CFS) = 6.77

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.45 HALFSTREET FLOOD WIDTH(FEET) = 14.34
FLOW VELOCITY(FEET/SEC.) = 3.02 DEPTH*VELOCITY(FT*FT/SEC.) = 1.34
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 17.00 = 1457.00 FEET.

FLOW PROCESS FROM NODE 17.00 TO NODE 17.00 IS CODE = 81 A6

>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.903
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8880
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 2.50 SUBAREA RUNOFF(CFS) = 6.44
TOTAL AREA(ACRES) = 4.9 TOTAL RUNOFF(CFS) = 13.22
TC(MIN.) = 15.13

FLOW PROCESS FROM NODE 17.00 TO NODE 17.00 IS CODE = 81 A7

>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.903
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7805
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 6.00 SUBAREA RUNOFF(CFS) = 13.59

TOTAL AREA(ACRES) = 10.9 TOTAL RUNOFF(CFS) = 26.81
TC(MIN.) = 15.13

FLOW PROCESS FROM NODE 17.00 TO NODE 17.00 IS CODE = 81 A8

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.903
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8880
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.10 SUBAREA RUNOFF(CFS) = 2.84
TOTAL AREA(ACRES) = 12.0 TOTAL RUNOFF(CFS) = 29.65
TC(MIN.) = 15.13

FLOW PROCESS FROM NODE 17.00 TO NODE 17.00 IS CODE = 81 A9

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.903
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8880
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.90 SUBAREA RUNOFF(CFS) = 2.32
TOTAL AREA(ACRES) = 12.9 TOTAL RUNOFF(CFS) = 31.97
TC(MIN.) = 15.13

FLOW PROCESS FROM NODE 17.00 TO NODE 18.00 IS CODE = 51 A10

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1269.30 DOWNSTREAM(FEET) = 1263.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 930.00 CHANNEL SLOPE = 0.0068
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 50.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.370
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7579
SOIL CLASSIFICATION IS "D"
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 43.25
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.95
AVERAGE FLOW DEPTH(FEET) = 0.67 TRAVEL TIME(MIN.) = 7.96
Tc(MIN.) = 23.09
SUBAREA AREA(ACRES) = 12.50 SUBAREA RUNOFF(CFS) = 22.46
TOTAL AREA(ACRES) = 25.4 PEAK FLOW RATE(CFS) = 54.42

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.72 FLOW VELOCITY(FEET/SEC.) = 2.09
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 18.00 = 2387.00 FEET.

FLOW PROCESS FROM NODE 18.00 TO NODE 14.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1257.40 DOWNSTREAM(FEET) = 1256.10
 FLOW LENGTH(FEET) = 97.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 33.0 INCH PIPE IS 24.8 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 11.39
 ESTIMATED PIPE DIAMETER(INCH) = 33.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 54.42
 PIPE TRAVEL TIME(MIN.) = 0.14 Tc(MIN.) = 23.24
 LONGEST FLOWPATH FROM NODE 15.00 TO NODE 14.00 = 2484.00 FEET.

 FLOW PROCESS FROM NODE 14.00 TO NODE 14.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
 >>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
 CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
 TIME OF CONCENTRATION(MIN.) = 23.24
 RAINFALL INTENSITY(INCH/HR) = 2.36
 TOTAL STREAM AREA(ACRES) = 25.40
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 54.42

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	35.21	15.90	2.834	13.70
2	54.42	23.24	2.363	25.40

*****WARNING*****
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	72.45	15.90	2.834
2	83.78	23.24	2.363

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:
 PEAK FLOW RATE(CFS) = 83.78 Tc(MIN.) = 23.24
 TOTAL AREA(ACRES) = 39.1
 LONGEST FLOWPATH FROM NODE 15.00 TO NODE 14.00 = 2484.00 FEET.

 FLOW PROCESS FROM NODE 14.00 TO NODE 19.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1256.10 DOWNSTREAM(FEET) = 1256.00
 FLOW LENGTH(FEET) = 70.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 60.0 INCH PIPE IS 43.4 INCHES

PIPE-FLOW VELOCITY(FEET/SEC.) = 5.51
ESTIMATED PIPE DIAMETER(INCH) = 60.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 83.78
PIPE TRAVEL TIME(MIN.) = 0.21 Tc(MIN.) = 23.45
LONGEST FLOWPATH FROM NODE 15.00 TO NODE 19.00 = 2554.00 FEET.

FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 81 A11

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.353
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8857
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 3.50 SUBAREA RUNOFF(CFS) = 7.29
TOTAL AREA(ACRES) = 42.6 TOTAL RUNOFF(CFS) = 91.07
TC(MIN.) = 23.45

FLOW PROCESS FROM NODE 19.00 TO NODE 20.00 IS CODE = 51 A12

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1256.00 DOWNSTREAM(FEET) = 1255.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 130.00 CHANNEL SLOPE = 0.0077
CHANNEL BASE(FEET) = 0.00 "Z" FACTOR = 5.000
MANNING'S FACTOR = 0.030 MAXIMUM DEPTH(FEET) = 2.00

==>>WARNING: FLOW IN CHANNEL EXCEEDS CHANNEL
CAPACITY(NORMAL DEPTH EQUAL TO SPECIFIED MAXIMUM
ALLOWABLE DEPTH).
AS AN APPROXIMATION, FLOWDEPTH IS SET AT MAXIMUM
ALLOWABLE DEPTH AND IS USED FOR TRAVELTIME CALCULATIONS.

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.333
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7560
SOIL CLASSIFICATION IS "D"
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 104.84
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.24
AVERAGE FLOW DEPTH(FEET) = 2.00 TRAVEL TIME(MIN.) = 0.41
Tc(MIN.) = 23.86
SUBAREA AREA(ACRES) = 15.60 SUBAREA RUNOFF(CFS) = 27.52
TOTAL AREA(ACRES) = 58.2 PEAK FLOW RATE(CFS) = 118.59

==>>WARNING: FLOW IN CHANNEL EXCEEDS CHANNEL
CAPACITY(NORMAL DEPTH EQUAL TO SPECIFIED MAXIMUM
ALLOWABLE DEPTH).
AS AN APPROXIMATION, FLOWDEPTH IS SET AT MAXIMUM
ALLOWABLE DEPTH AND IS USED FOR TRAVELTIME CALCULATIONS.

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 2.00 FLOW VELOCITY(FEET/SEC.) = 5.93

==>FLOWDEPTH EXCEEDS MAXIMUM ALLOWABLE DEPTH

LONGEST FLOWPATH FROM NODE 15.00 TO NODE 20.00 = 2684.00 FEET.

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 21 B1

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 205.00
UPSTREAM ELEVATION(FEET) = 1279.00
DOWNSTREAM ELEVATION(FEET) = 1276.00
ELEVATION DIFFERENCE(FEET) = 3.00
TC = 0.303*[(205.00**3)/(3.00)]**.2 = 5.932
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.547
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8920
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 6.49
TOTAL AREA(ACRES) = 1.60 TOTAL RUNOFF(CFS) = 6.49

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 51 B2

>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1276.00 DOWNSTREAM(FEET) = 1267.00
CHANNEL LENGTH THRU SUBAREA(FEET) = 391.00 CHANNEL SLOPE = 0.0230
CHANNEL BASE(FEET) = 1000.00 "Z" FACTOR = 99.900
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.968
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8883
SOIL CLASSIFICATION IS "D"
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.51
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 0.77
AVERAGE FLOW DEPTH(FEET) = 0.01 TRAVEL TIME(MIN.) = 8.51
Tc(MIN.) = 14.44
SUBAREA AREA(ACRES) = 2.20 SUBAREA RUNOFF(CFS) = 5.80
TOTAL AREA(ACRES) = 3.8 PEAK FLOW RATE(CFS) = 12.29

END OF SUBAREA CHANNEL FLOW HYDRAULICS:
DEPTH(FEET) = 0.01 FLOW VELOCITY(FEET/SEC.) = 0.99
LONGEST FLOWPATH FROM NODE 21.00 TO NODE 23.00 = 596.00 FEET.

FLOW PROCESS FROM NODE 10.00 TO NODE 30.00 IS CODE = 21 C1

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 255.00
UPSTREAM ELEVATION(FEET) = 1276.00
DOWNSTREAM ELEVATION(FEET) = 1274.00
ELEVATION DIFFERENCE(FEET) = 2.00

TC = 0.303*[(255.00**3)/(2.00)]**.2 = 7.333
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.107
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8912
 SOIL CLASSIFICATION IS "D"
 SUBAREA RUNOFF(CFS) = 1.83
 TOTAL AREA(ACRES) = 0.50 TOTAL RUNOFF(CFS) = 1.83

FLOW PROCESS FROM NODE 30.00 TO NODE 31.00 IS CODE = 61 *C2*

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STANDARD CURB SECTION USED)<<<<<

UPSTREAM ELEVATION(FEET) = 1274.00 DOWNSTREAM ELEVATION(FEET) = 1270.00
 STREET LENGTH(FEET) = 357.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 40.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 35.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.79
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.35
 HALFSTREET FLOOD WIDTH(FEET) = 9.76
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.44
 PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.86
 STREET FLOW TRAVEL TIME(MIN.) = 2.44 Tc(MIN.) = 9.77
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.580
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8901
 SOIL CLASSIFICATION IS "D"
 SUBAREA AREA(ACRES) = 0.60 SUBAREA RUNOFF(CFS) = 1.91
 TOTAL AREA(ACRES) = 1.1 PEAK FLOW RATE(CFS) = 3.74

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.38 HALFSTREET FLOOD WIDTH(FEET) = 11.17
 FLOW VELOCITY(FEET/SEC.) = 2.61 DEPTH*VELOCITY(FT*FT/SEC.) = 0.99
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 612.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 81 *C3*

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.580
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8901
 SOIL CLASSIFICATION IS "D"
 SUBAREA AREA(ACRES) = 1.90 SUBAREA RUNOFF(CFS) = 6.05
 TOTAL AREA(ACRES) = 3.0 TOTAL RUNOFF(CFS) = 9.80
 TC(MIN.) = 9.77

FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 61 *C4*

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1270.00 DOWNSTREAM ELEVATION(FEET) = 1264.50
STREET LENGTH(FEET) = 575.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 20.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 11.05
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.52
HALFSTREET FLOOD WIDTH(FEET) = 18.21
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.15
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.65
STREET FLOW TRAVEL TIME(MIN.) = 3.04 Tc(MIN.) = 12.81
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.144
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8889
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.90 SUBAREA RUNOFF(CFS) = 2.52
TOTAL AREA(ACRES) = 3.9 PEAK FLOW RATE(CFS) = 12.31

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.54 HALFSTREET FLOOD WIDTH(FEET) = 18.98
FLOW VELOCITY(FEET/SEC.) = 3.25 DEPTH*VELOCITY(FT*FT/SEC.) = 1.75
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 32.00 = 1187.00 FEET.

FLOW PROCESS FROM NODE 32.00 TO NODE 32.00 IS CODE = 81 C5

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.144
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8889
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.10 SUBAREA RUNOFF(CFS) = 3.07
TOTAL AREA(ACRES) = 5.0 TOTAL RUNOFF(CFS) = 15.39
TC(MIN.) = 12.81

FLOW PROCESS FROM NODE 32.00 TO NODE 32.00 IS CODE = 81 C6

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.144
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8889
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 5.90 SUBAREA RUNOFF(CFS) = 16.49
TOTAL AREA(ACRES) = 10.9 TOTAL RUNOFF(CFS) = 31.87
TC(MIN.) = 12.81

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1259.00 DOWNSTREAM(FEET) = 1257.00
FLOW LENGTH(FEET) = 54.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.83
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 31.87
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 12.87
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1241.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 81 C7

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.137
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8888
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 3.90 SUBAREA RUNOFF(CFS) = 10.87
TOTAL AREA(ACRES) = 14.8 TOTAL RUNOFF(CFS) = 42.75
TC(MIN.) = 12.87

FLOW PROCESS FROM NODE 33.00 TO NODE 80.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1257.00 DOWNSTREAM(FEET) = 1238.70
FLOW LENGTH(FEET) = 810.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 20.8 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.98
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 42.75
PIPE TRAVEL TIME(MIN.) = 1.04 Tc(MIN.) = 13.91
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 80.00 = 2051.00 FEET.

FLOW PROCESS FROM NODE 80.00 TO NODE 80.00 IS CODE = 81 F1

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.022
UNDEVELOPED WATERSHED RUNOFF COEFFICIENT = .7846
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 10.10 SUBAREA RUNOFF(CFS) = 23.95
TOTAL AREA(ACRES) = 24.9 TOTAL RUNOFF(CFS) = 66.70
TC(MIN.) = 13.91

FLOW PROCESS FROM NODE 40.00 TO NODE 41.00 IS CODE = 21 D1

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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=====
          ASSUMED INITIAL SUBAREA UNIFORM
          DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 192.00
UPSTREAM ELEVATION(FEET) = 1277.00
DOWNSTREAM ELEVATION(FEET) = 1275.00
ELEVATION DIFFERENCE(FEET) = 2.00
TC = 0.303*[(192.00**3)/(2.00)]**.2 = 6.185
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.456
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8918
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 3.58
TOTAL AREA(ACRES) = 0.90 TOTAL RUNOFF(CFS) = 3.58

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FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 61 D2

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>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

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=====
UPSTREAM ELEVATION(FEET) = 1275.00 DOWNSTREAM ELEVATION(FEET) = 1270.40
STREET LENGTH(FEET) = 942.00 CURB HEIGHT(INCHES) = 6.0
STREET HALFWIDTH(FEET) = 20.00

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DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

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SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

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**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.73
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
NOTE: STREET FLOW EXCEEDS TOP OF CURB.
THE FOLLOWING STREET FLOW RESULTS ARE BASED ON THE ASSUMPTION
THAT NEGLIBLE FLOW OCCURS OUTSIDE OF THE STREET CHANNEL.
THAT IS, ALL FLOW ALONG THE PARKWAY, ETC., IS NEGLECTED.
STREET FLOW DEPTH(FEET) = 0.53
HALFSTREET FLOOD WIDTH(FEET) = 19.96
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.37
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.25
STREET FLOW TRAVEL TIME(MIN.) = 6.62 Tc(MIN.) = 12.81
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.144
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8889
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 4.40 SUBAREA RUNOFF(CFS) = 12.30
TOTAL AREA(ACRES) = 5.3 PEAK FLOW RATE(CFS) = 15.87

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END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.53 HALFSTREET FLOOD WIDTH(FEET) = 20.00
FLOW VELOCITY(FEET/SEC.) = 2.37 DEPTH*VELOCITY(FT*FT/SEC.) = 1.25
*NOTE: INITIAL SUBAREA NOMOGRAPH WITH SUBAREA PARAMETERS,
AND L = 942.0 FT WITH ELEVATION-DROP = 4.6 FT, IS 11.9 CFS,
WHICH EXCEEDS THE TOP-OF-CURB STREET CAPACITY AT NODE 42.00
LONGEST FLOWPATH FROM NODE 40.00 TO NODE 42.00 = 1134.00 FEET.

```

FLOW PROCESS FROM NODE 50.00 TO NODE 51.00 IS CODE = 21 E1

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL

TC = $K * [(LENGTH**3) / (ELEVATION CHANGE)]**.2$
INITIAL SUBAREA FLOW-LENGTH(FEET) = 116.00
UPSTREAM ELEVATION(FEET) = 1276.00
DOWNSTREAM ELEVATION(FEET) = 1274.00
ELEVATION DIFFERENCE(FEET) = 2.00
TC = $0.303 * [(116.00**3) / (2.00)]**.2 = 4.571$
COMPUTED TIME OF CONCENTRATION INCREASED TO 5 MIN.
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.935
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8926
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 3.08
TOTAL AREA(ACRES) = 0.70 TOTAL RUNOFF(CFS) = 3.08

FLOW PROCESS FROM NODE 51.00 TO NODE 52.00 IS CODE = 91 E2

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<

=====

UPSTREAM NODE ELEVATION(FEET) = 1274.00
DOWNSTREAM NODE ELEVATION(FEET) = 1271.40
CHANNEL LENGTH THRU SUBAREA(FEET) = 334.00
"V" GUTTER WIDTH(FEET) = 3.00 GUTTER HIKE(FEET) = 0.170
PAVEMENT LIP(FEET) = 0.040 MANNING'S N = .0150
PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.01000
MAXIMUM DEPTH(FEET) = 1.00
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.062
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8911
SOIL CLASSIFICATION IS "D"
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 9.62
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 2.22
AVERAGE FLOW DEPTH(FEET) = 0.39 FLOOD WIDTH(FEET) = 39.88
"V" GUTTER FLOW TRAVEL TIME(MIN.) = 2.50 Tc(MIN.) = 7.50
SUBAREA AREA(ACRES) = 3.60 SUBAREA RUNOFF(CFS) = 13.03
TOTAL AREA(ACRES) = 4.3 PEAK FLOW RATE(CFS) = 16.12

END OF SUBAREA "V" GUTTER HYDRAULICS:
DEPTH(FEET) = 0.44 FLOOD WIDTH(FEET) = 49.75
FLOW VELOCITY(FEET/SEC.) = 2.46 DEPTH*VELOCITY(FT*FT/SEC) = 1.09
LONGEST FLOWPATH FROM NODE 50.00 TO NODE 52.00 = 450.00 FEET.

=====

END OF STUDY SUMMARY:
TOTAL AREA(ACRES) = 4.3 TC(MIN.) = 7.50
PEAK FLOW RATE(CFS) = 16.12

=====

END OF RATIONAL METHOD ANALYSIS

APPENDIX B: PROPOSED 100-YEAR HYDROLOGY CALCULATIONS

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM BASED ON
RIVERSIDE COUNTY FLOOD CONTROL & WATER CONSERVATION DISTRICT
(RCFC&WCD) 1978 HYDROLOGY MANUAL
(c) Copyright 1982-2007 Advanced Engineering Software (aes)
(Rational Tabling Version 7.0D)
Release Date: 06/01/2007 License ID 1557

Analysis prepared by:

***** DESCRIPTION OF STUDY *****
* PROPOSED HYDROLOGY ANALYSIS FOR DIAMOND CENTER *
* 100-YEAR RUN *
* BY KAM 022609 *

FILE NAME: DIAMOND.DAT
TIME/DATE OF STUDY: 12:21 03/11/2009

USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:

USER SPECIFIED STORM EVENT(YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 18.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
10-YEAR STORM 10-MINUTE INTENSITY(INCH/HOUR) = 2.320
10-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 0.980
100-YEAR STORM 10-MINUTE INTENSITY(INCH/HOUR) = 3.540
100-YEAR STORM 60-MINUTE INTENSITY(INCH/HOUR) = 1.500
SLOPE OF 10-YEAR INTENSITY-DURATION CURVE = 0.4809628
SLOPE OF 100-YEAR INTENSITY-DURATION CURVE = 0.4792280
COMPUTED RAINFALL INTENSITY DATA:

STORM EVENT = 100.00 1-HOUR INTENSITY(INCH/HOUR) = 1.500
SLOPE OF INTENSITY DURATION CURVE = 0.4792

RCFC&WCD HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD
NOTE: COMPUTE CONFLUENCE VALUES ACCORDING TO RCFC&WCD HYDROLOGY MANUAL
AND IGNORE OTHER CONFLUENCE COMBINATIONS FOR DOWNSTREAM ANALYSES

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT-/PARK- SIDE / SIDE/ WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP (FT)	MANNING HIKE (FT)	FACTOR (n)	
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

FLOW PROCESS FROM NODE 10.00 TO NODE 11.00 IS CODE = 21 A1

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL

TC = $K * [(LENGTH**3) / (ELEVATION CHANGE)]**0.2$
INITIAL SUBAREA FLOW-LENGTH(FEET) = 310.00
UPSTREAM ELEVATION(FEET) = 1276.50
DOWNSTREAM ELEVATION(FEET) = 1273.50
ELEVATION DIFFERENCE(FEET) = 3.00
TC = $0.303 * [(310.00**3) / (3.00)]**0.2 = 7.603$
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.037
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8911
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 1.44
TOTAL AREA(ACRES) = 0.40 TOTAL RUNOFF(CFS) = 1.44

FLOW PROCESS FROM NODE 11.00 TO NODE 12.00 IS CODE = 61 A2

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1273.50 DOWNSTREAM ELEVATION(FEET) = 1270.00
STREET LENGTH(FEET) = 496.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 25.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.61
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.37
HALFSTREET FLOOD WIDTH(FEET) = 10.56
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.00
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.74
STREET FLOW TRAVEL TIME(MIN.) = 4.13 Tc(MIN.) = 11.73
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.279
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8893
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.80 SUBAREA RUNOFF(CFS) = 2.33
TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 3.77

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.41 HALFSTREET FLOOD WIDTH(FEET) = 12.42
FLOW VELOCITY(FEET/SEC.) = 2.18 DEPTH*VELOCITY(FT*FT/SEC.) = 0.89
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 12.00 = 806.00 FEET.

FLOW PROCESS FROM NODE 12.00 TO NODE 12.00 IS CODE = 81 A3

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.279
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8893
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 4.50 SUBAREA RUNOFF(CFS) = 13.12
TOTAL AREA(ACRES) = 5.7 TOTAL RUNOFF(CFS) = 16.89
TC(MIN.) = 11.73

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*****
FLOW PROCESS FROM NODE 12.00 TO NODE 13.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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=====
ELEVATION DATA: UPSTREAM(FEET) = 1266.00 DOWNSTREAM(FEET) = 1265.80
FLOW LENGTH(FEET) = 42.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 18.5 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.83
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 16.89
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 11.85
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 848.00 FEET.

```

```

*****
FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1
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```

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

```

```

=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 11.85
RAINFALL INTENSITY(INCH/HR) = 3.26
TOTAL STREAM AREA(ACRES) = 5.70
PEAK FLOW RATE(CFS) AT CONFLUENCE = 16.89

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*****
FLOW PROCESS FROM NODE 10.00 TO NODE 14.00 IS CODE = 21 A4
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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=====
ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 330.00
UPSTREAM ELEVATION(FEET) = 1276.50
DOWNSTREAM ELEVATION(FEET) = 1273.50
ELEVATION DIFFERENCE(FEET) = 3.00
TC = 0.303*[(330.00**3)/(3.00)]**.2 = 7.894
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.965
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8909
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 2.12
TOTAL AREA(ACRES) = 0.60 TOTAL RUNOFF(CFS) = 2.12

```

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*****
FLOW PROCESS FROM NODE 14.00 TO NODE 15.00 IS CODE = 61 A5
-----

```

>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1273.50 DOWNSTREAM ELEVATION(FEET) = 1270.00
STREET LENGTH(FEET) = 500.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 25.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.43
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.40
HALFSTREET FLOOD WIDTH(FEET) = 11.93
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.13
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.84
STREET FLOW TRAVEL TIME(MIN.) = 3.91 Tc(MIN.) = 11.81
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.269
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8892
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.90 SUBAREA RUNOFF(CFS) = 2.62
TOTAL AREA(ACRES) = 1.5 PEAK FLOW RATE(CFS) = 4.74

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.43 HALFSTREET FLOOD WIDTH(FEET) = 13.68
FLOW VELOCITY(FEET/SEC.) = 2.30 DEPTH*VELOCITY(FT*FT/SEC.) = 0.99
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 15.00 = 830.00 FEET.

FLOW PROCESS FROM NODE 15.00 TO NODE 15.00 IS CODE = 81 AB

>>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.269
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8892
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 2.80 SUBAREA RUNOFF(CFS) = 8.14
TOTAL AREA(ACRES) = 4.3 TOTAL RUNOFF(CFS) = 12.88
TC(MIN.) = 11.81

FLOW PROCESS FROM NODE 15.00 TO NODE 13.00 IS CODE = 31

>>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<
>>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1266.00 DOWNSTREAM(FEET) = 1265.80
FLOW LENGTH(FEET) = 34.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.7 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.92
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 12.88
PIPE TRAVEL TIME(MIN.) = 0.10 Tc(MIN.) = 11.90

LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 864.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 13.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 11.90
RAINFALL INTENSITY(INCH/HR) = 3.26
TOTAL STREAM AREA(ACRES) = 4.30
PEAK FLOW RATE(CFS) AT CONFLUENCE = 12.88

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	16.89	11.85	3.263	5.70
2	12.88	11.90	3.257	4.30

*****WARNING*****

IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	29.71	11.85	3.263
2	29.73	11.90	3.257

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 29.71 Tc(MIN.) = 11.85
TOTAL AREA(ACRES) = 10.0
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 13.00 = 864.00 FEET.

FLOW PROCESS FROM NODE 13.00 TO NODE 16.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1265.80 DOWNSTREAM(FEET) = 1264.40
FLOW LENGTH(FEET) = 432.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 36.0 INCH PIPE IS 24.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.82
ESTIMATED PIPE DIAMETER(INCH) = 36.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 29.71
PIPE TRAVEL TIME(MIN.) = 1.24 Tc(MIN.) = 13.09
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 16.00 = 1296.00 FEET.

```

FLOW PROCESS FROM NODE      16.00 TO NODE      16.00 IS CODE =   1
-----
>>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
=====
TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:
TIME OF CONCENTRATION(MIN.) = 13.09
RAINFALL INTENSITY(INCH/HR) = 3.11
TOTAL STREAM AREA(ACRES) = 10.00
PEAK FLOW RATE(CFS) AT CONFLUENCE = 29.71

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*****
FLOW PROCESS FROM NODE      17.00 TO NODE      18.00 IS CODE = 21  A7
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>>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
          ASSUMED INITIAL SUBAREA UNIFORM
          DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 215.00
UPSTREAM ELEVATION(FEET) = 1284.00
DOWNSTREAM ELEVATION(FEET) = 1283.00
ELEVATION DIFFERENCE(FEET) = 1.00
TC = 0.303*[( 215.00**3)/( 1.00)]**.2 = 7.604
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.037
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8911
SOIL CLASSIFICATION IS "D"
SUBAREA RUNOFF(CFS) = 0.72
TOTAL AREA(ACRES) = 0.20 TOTAL RUNOFF(CFS) = 0.72

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*****
FLOW PROCESS FROM NODE      18.00 TO NODE      19.00 IS CODE = 61  AB
-----

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>>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>>(STANDARD CURB SECTION USED)<<<<<
=====
UPSTREAM ELEVATION(FEET) = 1283.00 DOWNSTREAM ELEVATION(FEET) = 1271.00
STREET LENGTH(FEET) = 1034.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 20.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 15.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

```

```

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.03
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.32
HALFSTREET FLOOD WIDTH(FEET) = 8.29
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.31
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.75
STREET FLOW TRAVEL TIME(MIN.) = 7.46 Tc(MIN.) = 15.06
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.909
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8881
SOIL CLASSIFICATION IS "D"

```

SUBAREA AREA(ACRES) = 1.00 SUBAREA RUNOFF(CFS) = 2.58
TOTAL AREA(ACRES) = 1.2 PEAK FLOW RATE(CFS) = 3.30

END OF SUBAREA STREET FLOW HYDRAULICS:

DEPTH(FEET) = 0.37 HALFSTREET FLOOD WIDTH(FEET) = 10.47
FLOW VELOCITY(FEET/SEC.) = 2.57 DEPTH*VELOCITY(FT*FT/SEC.) = 0.94
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 19.00 = 1249.00 FEET.

FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 81 A9

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.909
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8881
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.00 SUBAREA RUNOFF(CFS) = 2.58
TOTAL AREA(ACRES) = 2.2 TOTAL RUNOFF(CFS) = 5.89
TC(MIN.) = 15.06

FLOW PROCESS FROM NODE 19.00 TO NODE 19.00 IS CODE = 81 A10

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.909
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8881
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 8.60 SUBAREA RUNOFF(CFS) = 22.22
TOTAL AREA(ACRES) = 10.8 TOTAL RUNOFF(CFS) = 28.10
TC(MIN.) = 15.06

FLOW PROCESS FROM NODE 19.00 TO NODE 20.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1267.00 DOWNSTREAM(FEET) = 1266.00
FLOW LENGTH(FEET) = 70.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 27.0 INCH PIPE IS 17.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.03
ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 28.10
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 15.18
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 20.00 = 1319.00 FEET.

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 81 A11

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.898
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8880
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.60 SUBAREA RUNOFF(CFS) = 1.54
TOTAL AREA(ACRES) = 11.4 TOTAL RUNOFF(CFS) = 29.65

TC(MIN.) = 15.18

FLOW PROCESS FROM NODE 20.00 TO NODE 20.00 IS CODE = 81 A12

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.898
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8880
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.00 SUBAREA RUNOFF(CFS) = 2.57
TOTAL AREA(ACRES) = 12.4 TOTAL RUNOFF(CFS) = 32.22
TC(MIN.) = 15.18

FLOW PROCESS FROM NODE 20.00 TO NODE 16.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1266.00 DOWNSTREAM(FEET) = 1264.40
FLOW LENGTH(FEET) = 43.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 24.0 INCH PIPE IS 15.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 14.89
ESTIMATED PIPE DIAMETER(INCH) = 24.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 32.22
PIPE TRAVEL TIME(MIN.) = 0.05 Tc(MIN.) = 15.23
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 1362.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 16.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 15.23
RAINFALL INTENSITY(INCH/HR) = 2.89
TOTAL STREAM AREA(ACRES) = 12.40
PEAK FLOW RATE(CFS) AT CONFLUENCE = 32.22

** CONFLUENCE DATA **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	29.71	13.09	3.111	10.00
2	32.22	15.23	2.894	12.40

*****WARNING*****
IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED
ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA
WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	57.41	13.09	3.111
2	59.86	15.23	2.894

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 59.86 Tc (MIN.) = 15.23
TOTAL AREA (ACRES) = 22.4
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 16.00 = 1362.00 FEET.

FLOW PROCESS FROM NODE 16.00 TO NODE 21.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1264.40 DOWNSTREAM (FEET) = 1263.30
FLOW LENGTH (FEET) = 294.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 42.0 INCH PIPE IS 34.3 INCHES
PIPE-FLOW VELOCITY (FEET/SEC.) = 7.11
ESTIMATED PIPE DIAMETER (INCH) = 42.00 NUMBER OF PIPES = 1
PIPE-FLOW (CFS) = 59.86
PIPE TRAVEL TIME (MIN.) = 0.69 Tc (MIN.) = 15.92
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 21.00 = 1656.00 FEET.

FLOW PROCESS FROM NODE 21.00 TO NODE 21.00 IS CODE = 81 A13

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.833
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8878
SOIL CLASSIFICATION IS "D"
SUBAREA AREA (ACRES) = 3.60 SUBAREA RUNOFF (CFS) = 9.05
TOTAL AREA (ACRES) = 26.0 TOTAL RUNOFF (CFS) = 68.91
TC (MIN.) = 15.92

FLOW PROCESS FROM NODE 21.00 TO NODE 21.00 IS CODE = 81 A14

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 2.833
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8878
SOIL CLASSIFICATION IS "D"
SUBAREA AREA (ACRES) = 16.50 SUBAREA RUNOFF (CFS) = 41.50
TOTAL AREA (ACRES) = 42.5 TOTAL RUNOFF (CFS) = 110.41
TC (MIN.) = 15.92

FLOW PROCESS FROM NODE 21.00 TO NODE 22.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM (FEET) = 1263.30 DOWNSTREAM (FEET) = 1263.00

FLOW LENGTH(FEET) = 84.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 42.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.20
ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 110.41
PIPE TRAVEL TIME(MIN.) = 0.17 Tc(MIN.) = 16.09
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 22.00 = 1740.00 FEET.

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 81 A15

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.819
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8877
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.60 SUBAREA RUNOFF(CFS) = 1.50
TOTAL AREA(ACRES) = 43.1 TOTAL RUNOFF(CFS) = 111.91
TC(MIN.) = 16.09

FLOW PROCESS FROM NODE 22.00 TO NODE 22.00 IS CODE = 81 A16

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.819
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8877
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.50 SUBAREA RUNOFF(CFS) = 1.25
TOTAL AREA(ACRES) = 43.6 TOTAL RUNOFF(CFS) = 113.16
TC(MIN.) = 16.09

FLOW PROCESS FROM NODE 22.00 TO NODE 23.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1263.00 DOWNSTREAM(FEET) = 1262.00
FLOW LENGTH(FEET) = 292.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 57.0 INCH PIPE IS 41.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.24
ESTIMATED PIPE DIAMETER(INCH) = 57.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 113.16
PIPE TRAVEL TIME(MIN.) = 0.59 Tc(MIN.) = 16.68
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 23.00 = 2032.00 FEET.

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81 A17

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.770
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8876
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 5.20 SUBAREA RUNOFF(CFS) = 12.79
TOTAL AREA(ACRES) = 48.8 TOTAL RUNOFF(CFS) = 125.95

TC(MIN.) = 16.68

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81 A18

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.770
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8876
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.70 SUBAREA RUNOFF(CFS) = 4.18
TOTAL AREA(ACRES) = 50.5 TOTAL RUNOFF(CFS) = 130.13
TC(MIN.) = 16.68

FLOW PROCESS FROM NODE 23.00 TO NODE 23.00 IS CODE = 81 A19

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.770
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8876
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.40 SUBAREA RUNOFF(CFS) = 3.44
TOTAL AREA(ACRES) = 51.9 TOTAL RUNOFF(CFS) = 133.57
TC(MIN.) = 16.68

FLOW PROCESS FROM NODE 23.00 TO NODE 24.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1262.00 DOWNSTREAM(FEET) = 1261.30
FLOW LENGTH(FEET) = 174.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 57.0 INCH PIPE IS 44.4 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.02
ESTIMATED PIPE DIAMETER(INCH) = 57.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 133.57
PIPE TRAVEL TIME(MIN.) = 0.32 Tc(MIN.) = 17.00
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 24.00 = 2206.00 FEET.

FLOW PROCESS FROM NODE 24.00 TO NODE 24.00 IS CODE = 81 A20

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.745
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8875
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.80 SUBAREA RUNOFF(CFS) = 1.95
TOTAL AREA(ACRES) = 52.7 TOTAL RUNOFF(CFS) = 135.52
TC(MIN.) = 17.00

FLOW PROCESS FROM NODE 24.00 TO NODE 24.00 IS CODE = 81 A21

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

```

=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.745
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8875
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.80 SUBAREA RUNOFF(CFS) = 1.95
TOTAL AREA(ACRES) = 53.5 TOTAL RUNOFF(CFS) = 137.47
TC(MIN.) = 17.00

*****
FLOW PROCESS FROM NODE 24.00 TO NODE 25.00 IS CODE = 31
=====
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1261.30 DOWNSTREAM(FEET) = 1261.00
FLOW LENGTH(FEET) = 56.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 54.0 INCH PIPE IS 43.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 10.06
ESTIMATED PIPE DIAMETER(INCH) = 54.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 137.47
PIPE TRAVEL TIME(MIN.) = 0.09 Tc(MIN.) = 17.09
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 25.00 = 2262.00 FEET.

*****
FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 81 A22
=====
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.738
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8874
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 2.90 SUBAREA RUNOFF(CFS) = 7.05
TOTAL AREA(ACRES) = 56.4 TOTAL RUNOFF(CFS) = 144.51
TC(MIN.) = 17.09

*****
FLOW PROCESS FROM NODE 25.00 TO NODE 25.00 IS CODE = 81 A23
=====
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.738
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8874
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.50 SUBAREA RUNOFF(CFS) = 3.64
TOTAL AREA(ACRES) = 57.9 TOTAL RUNOFF(CFS) = 148.16
TC(MIN.) = 17.09

*****
FLOW PROCESS FROM NODE 25.00 TO NODE 26.00 IS CODE = 31
=====
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
=====
ELEVATION DATA: UPSTREAM(FEET) = 1261.00 DOWNSTREAM(FEET) = 1260.50
FLOW LENGTH(FEET) = 150.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 63.0 INCH PIPE IS 46.2 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.71

```

ESTIMATED PIPE DIAMETER(INCH) = 63.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 148.16
PIPE TRAVEL TIME(MIN.) = 0.29 Tc(MIN.) = 17.38
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 26.00 = 2412.00 FEET.

FLOW PROCESS FROM NODE 26.00 TO NODE 26.00 IS CODE = 81 A24

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.716
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8873
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.90 SUBAREA RUNOFF(CFS) = 2.17
TOTAL AREA(ACRES) = 58.8 TOTAL RUNOFF(CFS) = 150.33
TC(MIN.) = 17.38

FLOW PROCESS FROM NODE 26.00 TO NODE 26.00 IS CODE = 81 A25

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.716
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8873
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.20 SUBAREA RUNOFF(CFS) = 2.89
TOTAL AREA(ACRES) = 60.0 TOTAL RUNOFF(CFS) = 153.22
TC(MIN.) = 17.38

FLOW PROCESS FROM NODE 26.00 TO NODE 27.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1260.50 DOWNSTREAM(FEET) = 1259.80
FLOW LENGTH(FEET) = 193.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 63.0 INCH PIPE IS 45.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.08
ESTIMATED PIPE DIAMETER(INCH) = 63.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 153.22
PIPE TRAVEL TIME(MIN.) = 0.35 Tc(MIN.) = 17.74
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 27.00 = 2605.00 FEET.

FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 81 A26

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.690
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8872
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 3.50 SUBAREA RUNOFF(CFS) = 8.35
TOTAL AREA(ACRES) = 63.5 TOTAL RUNOFF(CFS) = 161.57
TC(MIN.) = 17.74

FLOW PROCESS FROM NODE 27.00 TO NODE 27.00 IS CODE = 81 A27

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.690
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8872
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.30 SUBAREA RUNOFF(CFS) = 3.10
TOTAL AREA(ACRES) = 64.8 TOTAL RUNOFF(CFS) = 164.67
TC(MIN.) = 17.74

FLOW PROCESS FROM NODE 27.00 TO NODE 28.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1259.80 DOWNSTREAM(FEET) = 1259.50
FLOW LENGTH(FEET) = 71.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 60.0 INCH PIPE IS 49.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.58
ESTIMATED PIPE DIAMETER(INCH) = 60.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 164.67
PIPE TRAVEL TIME(MIN.) = 0.12 Tc(MIN.) = 17.86
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 28.00 = 2676.00 FEET.

FLOW PROCESS FROM NODE 28.00 TO NODE 28.00 IS CODE = 81 A28

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.681
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8872
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.50 SUBAREA RUNOFF(CFS) = 3.57
TOTAL AREA(ACRES) = 66.3 TOTAL RUNOFF(CFS) = 168.24
TC(MIN.) = 17.86

FLOW PROCESS FROM NODE 28.00 TO NODE 29.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1259.50 DOWNSTREAM(FEET) = 1257.40
FLOW LENGTH(FEET) = 573.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 63.0 INCH PIPE IS 49.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 9.21
ESTIMATED PIPE DIAMETER(INCH) = 63.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 168.24
PIPE TRAVEL TIME(MIN.) = 1.04 Tc(MIN.) = 18.90
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 29.00 = 3249.00 FEET.

FLOW PROCESS FROM NODE 29.00 TO NODE 29.00 IS CODE = 81 A29

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

```
=====
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.609
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8869
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.30 SUBAREA RUNOFF(CFS) = 0.69
TOTAL AREA(ACRES) = 66.6 TOTAL RUNOFF(CFS) = 168.94
TC(MIN.) = 18.90
```

```
*****
FLOW PROCESS FROM NODE 29.00 TO NODE 29.00 IS CODE = 81 A30
-----
```

```
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
```

```
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.609
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8869
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 2.20 SUBAREA RUNOFF(CFS) = 5.09
TOTAL AREA(ACRES) = 68.8 TOTAL RUNOFF(CFS) = 174.03
TC(MIN.) = 18.90
```

```
*****
FLOW PROCESS FROM NODE 29.00 TO NODE 29.00 IS CODE = 81 A31
-----
```

```
>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<
=====
```

```
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 2.609
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8869
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.30 SUBAREA RUNOFF(CFS) = 0.69
TOTAL AREA(ACRES) = 69.1 TOTAL RUNOFF(CFS) = 174.72
TC(MIN.) = 18.90
```

```
*****
FLOW PROCESS FROM NODE 29.00 TO NODE 81.00 IS CODE = 31
-----
```

```
>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<
=====
```

```
ELEVATION DATA: UPSTREAM(FEET) = 1257.40 DOWNSTREAM(FEET) = 1238.70
FLOW LENGTH(FEET) = 1020.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 48.0 INCH PIPE IS 36.3 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 17.12
ESTIMATED PIPE DIAMETER(INCH) = 48.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 174.72
PIPE TRAVEL TIME(MIN.) = 0.99 Tc(MIN.) = 19.89
LONGEST FLOWPATH FROM NODE 17.00 TO NODE 81.00 = 4269.00 FEET.
```

```
*****
END OF SUB-AREA A
FLOW PROCESS FROM NODE 40.00 TO NODE 41.00 IS CODE = 21 B1
-----
```

```
>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
=====
```

```
ASSUMED INITIAL SUBAREA UNIFORM
DEVELOPMENT IS COMMERCIAL
TC = K*[(LENGTH**3)/(ELEVATION CHANGE)]**.2
INITIAL SUBAREA FLOW-LENGTH(FEET) = 306.00
UPSTREAM ELEVATION(FEET) = 1277.00
```

DOWNSTREAM ELEVATION(FEET) = 1274.10
 ELEVATION DIFFERENCE(FEET) = 2.90
 $TC = 0.303 * [(306.00^{**3}) / (2.90)]^{**2} = 7.595$
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.039
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8911
 SOIL CLASSIFICATION IS "D"
 SUBAREA RUNOFF(CFS) = 5.04
 TOTAL AREA(ACRES) = 1.40 TOTAL RUNOFF(CFS) = 5.04

 FLOW PROCESS FROM NODE 41.00 TO NODE 42.00 IS CODE = 91 **B2**

>>>>COMPUTE "V" GUTTER FLOW TRAVEL TIME THRU SUBAREA<<<<<

UPSTREAM NODE ELEVATION(FEET) = 1274.10
 DOWNSTREAM NODE ELEVATION(FEET) = 1270.50
 CHANNEL LENGTH THRU SUBAREA(FEET) = 455.00
 "V" GUTTER WIDTH(FEET) = 5.00 GUTTER HIKE(FEET) = 0.330
 PAVEMENT LIP(FEET) = 0.010 MANNING'S N = .0300
 PAVEMENT CROSSFALL(DECIMAL NOTATION) = 0.02000
 MAXIMUM DEPTH(FEET) = 1.00
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.216
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8891
 SOIL CLASSIFICATION IS "D"
 TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 7.90
 TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 1.64
 AVERAGE FLOW DEPTH(FEET) = 0.58 FLOOD WIDTH(FEET) = 28.53
 "V" GUTTER FLOW TRAVEL TIME(MIN.) = 4.62 T_c (MIN.) = 12.22
 SUBAREA AREA(ACRES) = 2.00 SUBAREA RUNOFF(CFS) = 5.72
 TOTAL AREA(ACRES) = 3.4 PEAK FLOW RATE(CFS) = 10.76

END OF SUBAREA "V" GUTTER HYDRAULICS:
 DEPTH(FEET) = 0.62 FLOOD WIDTH(FEET) = 33.29
 FLOW VELOCITY(FEET/SEC.) = 1.71 DEPTH*VELOCITY(FT*FT/SEC) = 1.06
 LONGEST FLOWPATH FROM NODE 40.00 TO NODE 42.00 = 761.00 FEET.

END OF SUB-AREA B

 FLOW PROCESS FROM NODE 10.00 TO NODE 30.00 IS CODE = 21 **C1**

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

ASSUMED INITIAL SUBAREA UNIFORM
 DEVELOPMENT IS COMMERCIAL
 $TC = K * [(LENGTH^{**3}) / (ELEVATION CHANGE)]^{**2}$
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 255.00
 UPSTREAM ELEVATION(FEET) = 1276.50
 DOWNSTREAM ELEVATION(FEET) = 1274.00
 ELEVATION DIFFERENCE(FEET) = 2.50
 $TC = 0.303 * [(255.00^{**3}) / (2.50)]^{**2} = 7.013$
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.196
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8914
 SOIL CLASSIFICATION IS "D"
 SUBAREA RUNOFF(CFS) = 1.87
 TOTAL AREA(ACRES) = 0.50 TOTAL RUNOFF(CFS) = 1.87

 FLOW PROCESS FROM NODE 30.00 TO NODE 31.00 IS CODE = 61 **C2**

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1274.00 DOWNSTREAM ELEVATION(FEET) = 1270.00
STREET LENGTH(FEET) = 357.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 25.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.84
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.36
HALFSTREET FLOOD WIDTH(FEET) = 9.85
AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.45
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.87
STREET FLOW TRAVEL TIME(MIN.) = 2.43 Tc(MIN.) = 9.44
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.639
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8902
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.60 SUBAREA RUNOFF(CFS) = 1.94
TOTAL AREA(ACRES) = 1.1 PEAK FLOW RATE(CFS) = 3.81

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.38 HALFSTREET FLOOD WIDTH(FEET) = 11.27
FLOW VELOCITY(FEET/SEC.) = 2.61 DEPTH*VELOCITY(FT*FT/SEC.) = 1.00
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 31.00 = 612.00 FEET.

FLOW PROCESS FROM NODE 31.00 TO NODE 31.00 IS CODE = 81 *C3*

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.639
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8902
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 1.90 SUBAREA RUNOFF(CFS) = 6.16
TOTAL AREA(ACRES) = 3.0 TOTAL RUNOFF(CFS) = 9.97
TC(MIN.) = 9.44

FLOW PROCESS FROM NODE 31.00 TO NODE 32.00 IS CODE = 61 *C4*

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>(STANDARD CURB SECTION USED)<<<<<

=====

UPSTREAM ELEVATION(FEET) = 1270.00 DOWNSTREAM ELEVATION(FEET) = 1266.50
STREET LENGTH(FEET) = 350.00 CURB HEIGHT(INCHES) = 8.0
STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 25.00
INSIDE STREET CROSSFALL(DECIMAL) = 0.020

OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curb) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 10.86
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
STREET FLOW DEPTH(FEET) = 0.52
HALFSTREET FLOOD WIDTH(FEET) = 17.89
AVERAGE FLOW VELOCITY(FEET/SEC.) = 3.21
PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 1.65
STREET FLOW TRAVEL TIME(MIN.) = 1.82 Tc(MIN.) = 11.26
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.344
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8894
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.60 SUBAREA RUNOFF(CFS) = 1.78
TOTAL AREA(ACRES) = 3.6 PEAK FLOW RATE(CFS) = 11.75

END OF SUBAREA STREET FLOW HYDRAULICS:
DEPTH(FEET) = 0.53 HALFSTREET FLOOD WIDTH(FEET) = 18.49
FLOW VELOCITY(FEET/SEC.) = 3.26 DEPTH*VELOCITY(FT*FT/SEC.) = 1.72
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 32.00 = 962.00 FEET.

FLOW PROCESS FROM NODE 32.00 TO NODE 32.00 IS CODE = 81 *CS*

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

=====

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.344
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8894
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 2.90 SUBAREA RUNOFF(CFS) = 8.63
TOTAL AREA(ACRES) = 6.5 TOTAL RUNOFF(CFS) = 20.38
TC(MIN.) = 11.26

FLOW PROCESS FROM NODE 32.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====

ELEVATION DATA: UPSTREAM(FEET) = 1257.50 DOWNSTREAM(FEET) = 1257.00
FLOW LENGTH(FEET) = 169.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 30.0 INCH PIPE IS 23.0 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 5.04
ESTIMATED PIPE DIAMETER(INCH) = 30.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 20.38
PIPE TRAVEL TIME(MIN.) = 0.56 Tc(MIN.) = 11.82
LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1131.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<

=====

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 1 ARE:

TIME OF CONCENTRATION(MIN.) = 11.82
 RAINFALL INTENSITY(INCH/HR) = 3.27
 TOTAL STREAM AREA(ACRES) = 6.50
 PEAK FLOW RATE(CFS) AT CONFLUENCE = 20.38

 FLOW PROCESS FROM NODE 34.00 TO NODE 35.00 IS CODE = 21 *C6*

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====
 ASSUMED INITIAL SUBAREA UNIFORM
 DEVELOPMENT IS COMMERCIAL
 TC = $K * [(LENGTH ** 3) / (ELEVATION CHANGE)] ** .2$
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 312.00
 UPSTREAM ELEVATION(FEET) = 1276.00
 DOWNSTREAM ELEVATION(FEET) = 1272.00
 ELEVATION DIFFERENCE(FEET) = 4.00
 TC = $0.303 * [(312.00 ** 3) / (4.00)] ** .2 = 7.206$
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.142
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8913
 SOIL CLASSIFICATION IS "D"
 SUBAREA RUNOFF(CFS) = 1.11
 TOTAL AREA(ACRES) = 0.30 TOTAL RUNOFF(CFS) = 1.11

 FLOW PROCESS FROM NODE 35.00 TO NODE 36.00 IS CODE = 61 *C7*

>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>(STANDARD CURB SECTION USED)<<<<<

=====
 UPSTREAM ELEVATION(FEET) = 1272.00 DOWNSTREAM ELEVATION(FEET) = 1264.50
 STREET LENGTH(FEET) = 687.00 CURB HEIGHT(INCHES) = 8.0
 STREET HALFWIDTH(FEET) = 30.00

DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 25.00
 INSIDE STREET CROSSFALL(DECIMAL) = 0.020
 OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020

SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 1
 Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) = 0.0150

**TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 2.26
 STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:
 STREET FLOW DEPTH(FEET) = 0.34
 HALFSTREET FLOOD WIDTH(FEET) = 8.86
 AVERAGE FLOW VELOCITY(FEET/SEC.) = 2.32
 PRODUCT OF DEPTH&VELOCITY(FT*FT/SEC.) = 0.78
 STREET FLOW TRAVEL TIME(MIN.) = 4.94 Tc(MIN.) = 12.14
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.225
 COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8891
 SOIL CLASSIFICATION IS "D"
 SUBAREA AREA(ACRES) = 0.80 SUBAREA RUNOFF(CFS) = 2.29
 TOTAL AREA(ACRES) = 1.1 PEAK FLOW RATE(CFS) = 3.40

END OF SUBAREA STREET FLOW HYDRAULICS:
 DEPTH(FEET) = 0.37 HALFSTREET FLOOD WIDTH(FEET) = 10.78
 FLOW VELOCITY(FEET/SEC.) = 2.52 DEPTH*VELOCITY(FT*FT/SEC.) = 0.94

LONGEST FLOWPATH FROM NODE 34.00 TO NODE 36.00 = 999.00 FEET.

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 81 CB

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.225
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8891
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 0.30 SUBAREA RUNOFF(CFS) = 0.86
TOTAL AREA(ACRES) = 1.4 TOTAL RUNOFF(CFS) = 4.26
TC(MIN.) = 12.14

FLOW PROCESS FROM NODE 36.00 TO NODE 36.00 IS CODE = 81 C9

>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.225
COMMERCIAL DEVELOPMENT RUNOFF COEFFICIENT = .8891
SOIL CLASSIFICATION IS "D"
SUBAREA AREA(ACRES) = 5.90 SUBAREA RUNOFF(CFS) = 16.92
TOTAL AREA(ACRES) = 7.3 TOTAL RUNOFF(CFS) = 21.18
TC(MIN.) = 12.14

FLOW PROCESS FROM NODE 36.00 TO NODE 33.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

ELEVATION DATA: UPSTREAM(FEET) = 1259.00 DOWNSTREAM(FEET) = 1257.00
FLOW LENGTH(FEET) = 54.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 21.0 INCH PIPE IS 13.1 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 13.42
ESTIMATED PIPE DIAMETER(INCH) = 21.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 21.18
PIPE TRAVEL TIME(MIN.) = 0.07 Tc(MIN.) = 12.21
LONGEST FLOWPATH FROM NODE 34.00 TO NODE 33.00 = 1053.00 FEET.

FLOW PROCESS FROM NODE 33.00 TO NODE 33.00 IS CODE = 1

>>>>DESIGNATE INDEPENDENT STREAM FOR CONFLUENCE<<<<<
>>>>AND COMPUTE VARIOUS CONFLUENCED STREAM VALUES<<<<<

TOTAL NUMBER OF STREAMS = 2
CONFLUENCE VALUES USED FOR INDEPENDENT STREAM 2 ARE:
TIME OF CONCENTRATION(MIN.) = 12.21
RAINFALL INTENSITY(INCH/HR) = 3.22
TOTAL STREAM AREA(ACRES) = 7.30
PEAK FLOW RATE(CFS) AT CONFLUENCE = 21.18

** CONFLUENCE DATA **

STREAM RUNOFF Tc INTENSITY AREA
NUMBER (CFS) (MIN.) (INCH/HOUR) (ACRE)

1	20.38	11.82	3.268	6.50
2	21.18	12.21	3.217	7.30

*****WARNING*****
 IN THIS COMPUTER PROGRAM, THE CONFLUENCE VALUE USED IS BASED
 ON THE RCFC&WCD FORMULA OF PLATE D-1 AS DEFAULT VALUE. THIS FORMULA
 WILL NOT NECESSARILY RESULT IN THE MAXIMUM VALUE OF PEAK FLOW.

RAINFALL INTENSITY AND TIME OF CONCENTRATION RATIO
 CONFLUENCE FORMULA USED FOR 2 STREAMS.

** PEAK FLOW RATE TABLE **

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	40.88	11.82	3.268
2	41.24	12.21	3.217

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE(CFS) = 41.24 Tc(MIN.) = 12.21
 TOTAL AREA(ACRES) = 13.8
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 33.00 = 1131.00 FEET.

 FLOW PROCESS FROM NODE 33.00 TO NODE 80.00 IS CODE = 31

>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<
 >>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 1257.00 DOWNSTREAM(FEET) = 1238.70
 FLOW LENGTH(FEET) = 810.00 MANNING'S N = 0.013
 DEPTH OF FLOW IN 27.0 INCH PIPE IS 20.2 INCHES
 PIPE-FLOW VELOCITY(FEET/SEC.) = 12.93
 ESTIMATED PIPE DIAMETER(INCH) = 27.00 NUMBER OF PIPES = 1
 PIPE-FLOW(CFS) = 41.24
 PIPE TRAVEL TIME(MIN.) = 1.04 Tc(MIN.) = 13.26
 LONGEST FLOWPATH FROM NODE 10.00 TO NODE 80.00 = 1941.00 FEET.

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END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 13.8 TC(MIN.) = 13.26
 PEAK FLOW RATE(CFS) = 41.24

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END OF SUB-AREA C

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END OF RATIONAL METHOD ANALYSIS