
Appendix K

Preliminary Hydrology Study

PRELIMINARY HYDROLOGY STUDY

FOR

Commercial Retail Lake Elsinore

**NWC Mountain St. & Lake St.,
Lake Elsinore, CA 92530**

**Prepared By:
PLUMP ENGINEERING, INC.
914 E. Katella Avenue
Anaheim, CA 92805**



This Drainage Report was prepared under my supervision:

By: _____

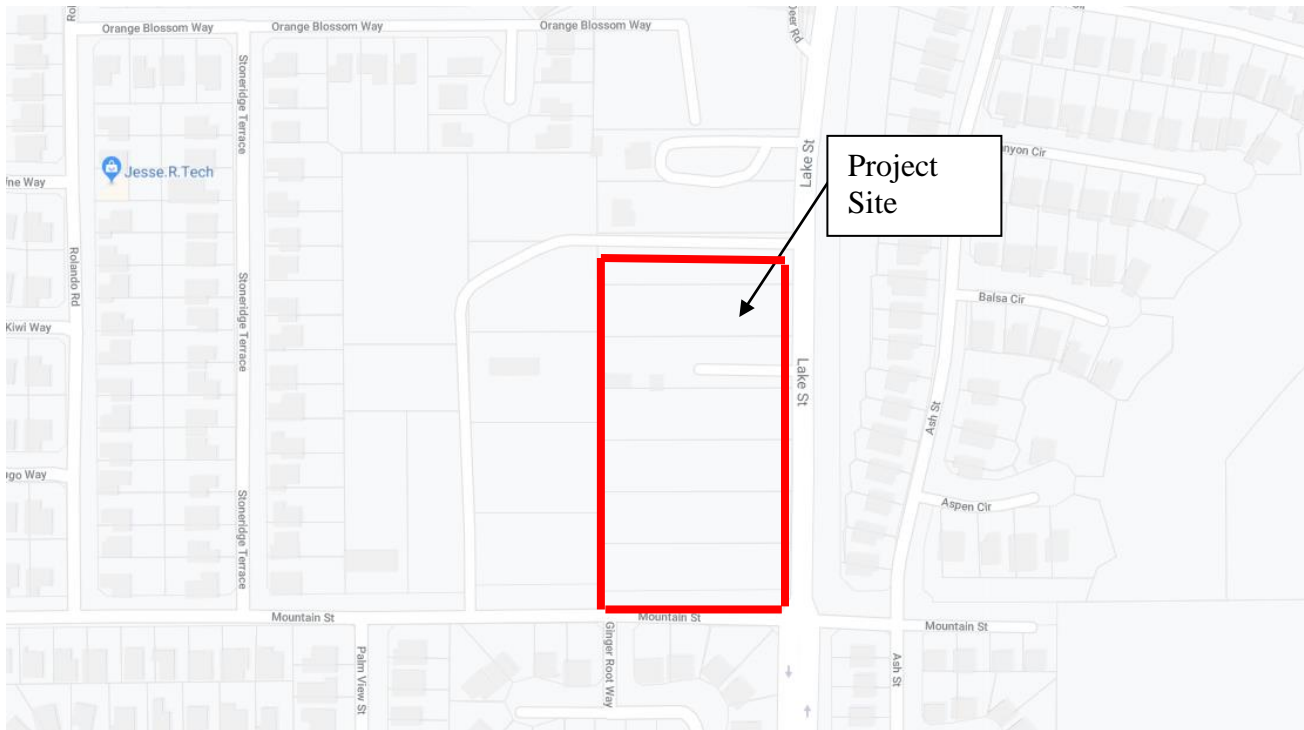
Date: 10/04/2019

INTRODUCTION

The site is located at the northwest corner of the intersection of Mountain Street and Lake Street in the City of Lake Elsinore, California 92530. The site is bounded to the west by undeveloped lot, to the north by a private property, to the east by Lake Street and to the south by Mountain Street. The general location of the site is shown on the Site Vicinity Map included on page 2 this report. The subject site is a rectangular shaped, Property size is approximately 5.630± acres with disturbed area of 5.630± acres, presently vacant and undeveloped. Ground surface cover consists of sparse amounts of native grass and weed growth located at the entire site. Overhead electrical power lines are located along the western and southern perimeter of the property. Overall site topography slopes downward to the southwest at a gradient of approximately less than 8.50 percent. There was estimated to be 20± feet of elevation differential across the overall subject site.

The preliminary site plan for the proposed development indicates that the site will be developed with a gas station with car wash and store, four (4) retail buildings, two (2) restaurant buildings with drive-thru, and five (5) trash enclosures . The new gas station canopy will be 4,089± SF, the four new retail buildings will be 13,200± SF, The two restaurants will be 5,840± SF, the car wash will be 3,159± SF, the store will be 3,400± SF, and the trash enclosures will cover 463± SF. The site plan indicates that the proposed buildings will generally be surrounded by asphaltic concrete with some areas of concrete flatwork. Several landscape planters are proposed to be located around the perimeter and within the parking lot of the site.

VICINITY MAP



HYDROLOGY ANALYSIS

Hydrologic calculations were performed in accordance with Riverside County Hydrology Manual (April 1978) guidelines. The Hydrology Manual was used to determine the existing and proposed peak flows for the, 2-year, 10-year and 25-year storms as well as the runoff volumes generated for , 2-year, 10-year and 25-year storms as well as the runoff volumes generated for 100-year storm event. Figures and Tables below are referenced to that Manual. The previous pre-development use is a vacant land with pervious cover.

RUNOFF FLOWS

Table 1: Rational Method Peak Flow

Tributary Area	DMA-1	DMA-2	Totals
Acreage, acres Pre (Post)	5.63 (2.38)	- (3.25)	5.63
Time of Concentration; Tc (min) (Post)	21 (9.5)	- (8.5)	21 9
2 year Pre- Develop. Runoff; Q ₂ (cfs)	0.03	-	0.03
2 year Post- Develop. Runoff; Q ₂ (cfs)	0.76	1.04	1.79
10 year Pre- Develop. Runoff; Q ₁₀ (cfs)	0.05	-	0.05
10 year Post- Develop. Runoff; Q ₁₀ (cfs)	1.25	1.70	2.95
25 year Pre- Develop. Runoff; Q ₂₅ (cfs)	0.06	-	0.06
25 year Post- Develop. Runoff; Q ₂₅ (cfs)	1.56	2.13	3.68

RUNOFF VOLUMES

Table 2: Estimated Storm Runoff Volumes

Tributary Area	Area 1	Area 2	Totals
Acreage (Pre Development) Acres	5.63	-	5.63
Acreage (Post Development) Acres	(2.38)	(3.25)	(5.63)
24-hr. Precipitation Depth(inches) P ₂₄ (2 yr)	2.35	2.35	-
24-hr. Precipitation Depth(inches) P ₂₄ (10 yr)	3.88	3.88	-
24-hr. Precipitation Depth(inches) P ₂₄ (100 yr)	6.25	6.25	-
CN (AMC II) Pre Development	75	-	-
CN (AMC II) Post Development	96	96	-

CN (AMC I) Pre Development	57	-	-
CN (AMC I) Post Development	78	78	-
CN (AMC III) Pre Development	88	-	-
CN (AMC III) Post Development	96	96	-
2 year Pre-Develop. Volume V_2 (Ac-ft)	0.04	-	-
2 year Post-Develop. Volume V_2 (Ac-ft)	0.137	0.188	0.325
10 year Pre-Develop. Volume V_{10} (Ac-ft)	1.228	-	-
10 year Post-Develop. Volume V_{10} (Ac-ft)	0.68	0.93	1.605
100 year Pre-Develop. Volume V_{100} (Ac-ft)	2.28	-	-
100 year Post-Develop. Volume V_{100} (Ac-ft)	1.15	1.56	2.710

CONCLUSION

The existing peak runoff from the project area was calculated to be 0.05 cfs, and 0.06 cfs for the 10-year and 25-year storms respectively. The proposed peak runoff from the project area after improvements was calculated to be 2.95 cfs, and 3.68 cfs for the 10-year and 25-year storms respectively. Therefore, outlet detention and retention will be necessary to replicate the pre-development condition in order to protect the downstream storm drains.

The existing runoff volumes from the project area was calculated to be 2.28 Ac-ft. for the 100-year storm and the proposed runoff volume from the project area after improvements was calculated to be 2.71 Ac-ft. for the 100-year storm. Post development condition is 16% higher than the pre development condition. Therefore, there are hydraulic conditions of concern (HCOC's) exist for this project. Excess amount will be retained and filtered on site via biofiltration with underdrain. HCOC Mitigation of the WQMP report will utilize accepted professional methodologies published by entities such as CASQA via biotreatment by means of biofiltration with underdrain.

Attachment A

Figures

GINGER ROOT WAY

MOUNTAIN STREET

LAKE STREET

LEGEND:

- LANDSCAPE AREA
- DRAINAGE FLOW DIRECTION
- DMA BOUNDARY
- PARCEL BOUNDARY
- DMA-X
X,XXX SF
XX.XX ac
- DMA INFORMATION

RUN-OFF FLOW

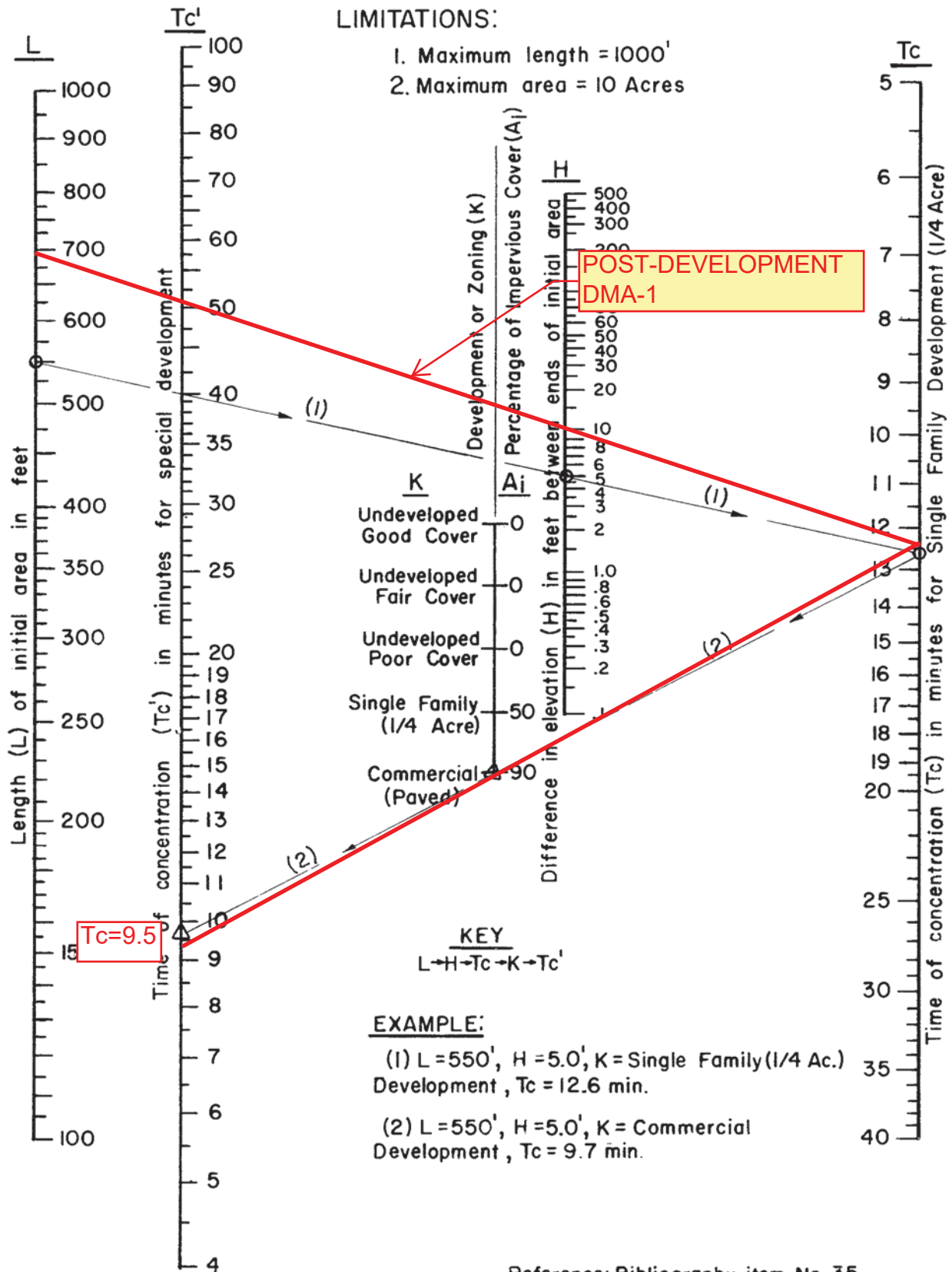
Return Frequency = 2 years							
POST-DEVELOPMENT							
Drainage Area	A (acres)	Soils Group	Coefficient C	T _c (Final) (min)	I ₁ hr (ln/hr)	Q (cfs)	
1	2.380	D	0.648	9.5	0.492	0.76	
2	3.250	D	0.648	8.5	0.492	1.04	
TOTAL						1.79	
Return Frequency = 10 years							
POST-DEVELOPMENT							
Drainage Area	A (acres)	Soils Group	Coefficient C	T _c (Final) (min)	I ₁ hr (ln/hr)	Q (cfs)	
1	2.380	D	0.648	9.5	0.808	1.25	
2	3.250	D	0.648	8.5	0.808	1.70	
TOTAL						2.95	
Return Frequency = 25 years							
POST-DEVELOPMENT							
Drainage Area	A (acres)	Soils Group	Coefficient C	T _c (Final) (min)	I ₁ hr (ln/hr)	Q (cfs)	
1	2.380	D	0.648	9.5	1.01	1.56	
2	3.250	D	0.648	8.5	1.01	2.13	
TOTAL						3.68	

RUN-OFF VOLUME

Return Frequency = 2 years										
POST-DEVELOPMENT										
Drainage Area	A (Acres)	Soils Group	CN AMC II	CN AMC I	S	I _a	P ₂₄	Y	V (Ac-ft)	
1	2.380	D	90	78	2.82	0.56	2.35	0.29	0.137	
2	3.250	D	90	78	2.82	0.56	2.35	0.29	0.188	
TOTAL									0.325	
Return Frequency = 10 years										
POST-DEVELOPMENT										
Drainage Area	A (acres)	Soils Group	CN AMC II	CN AMC III	S	I _a	P ₂₄	Y	V (Ac-ft)	
1	2.380	D	90	96	0.42	0.08	3.88	0.88	0.68	
2	3.250	D	90	96	0.42	0.08	3.88	0.88	0.93	
TOTAL									1.605	
Return Frequency = 100 years										
POST-DEVELOPMENT										
Drainage Area	A (acres)	Soils Group	CN AMC II	CN AMC III	S	I _a	P ₂₄	Y	V (Ac-ft)	
1	2.380	D	90	96	0.42	0.08	6.25	0.92	1.15	
2	3.250	D	90	96	0.42	0.08	6.25	0.92	1.56	
TOTAL									2.710	

30' 0 30' 60' 90'

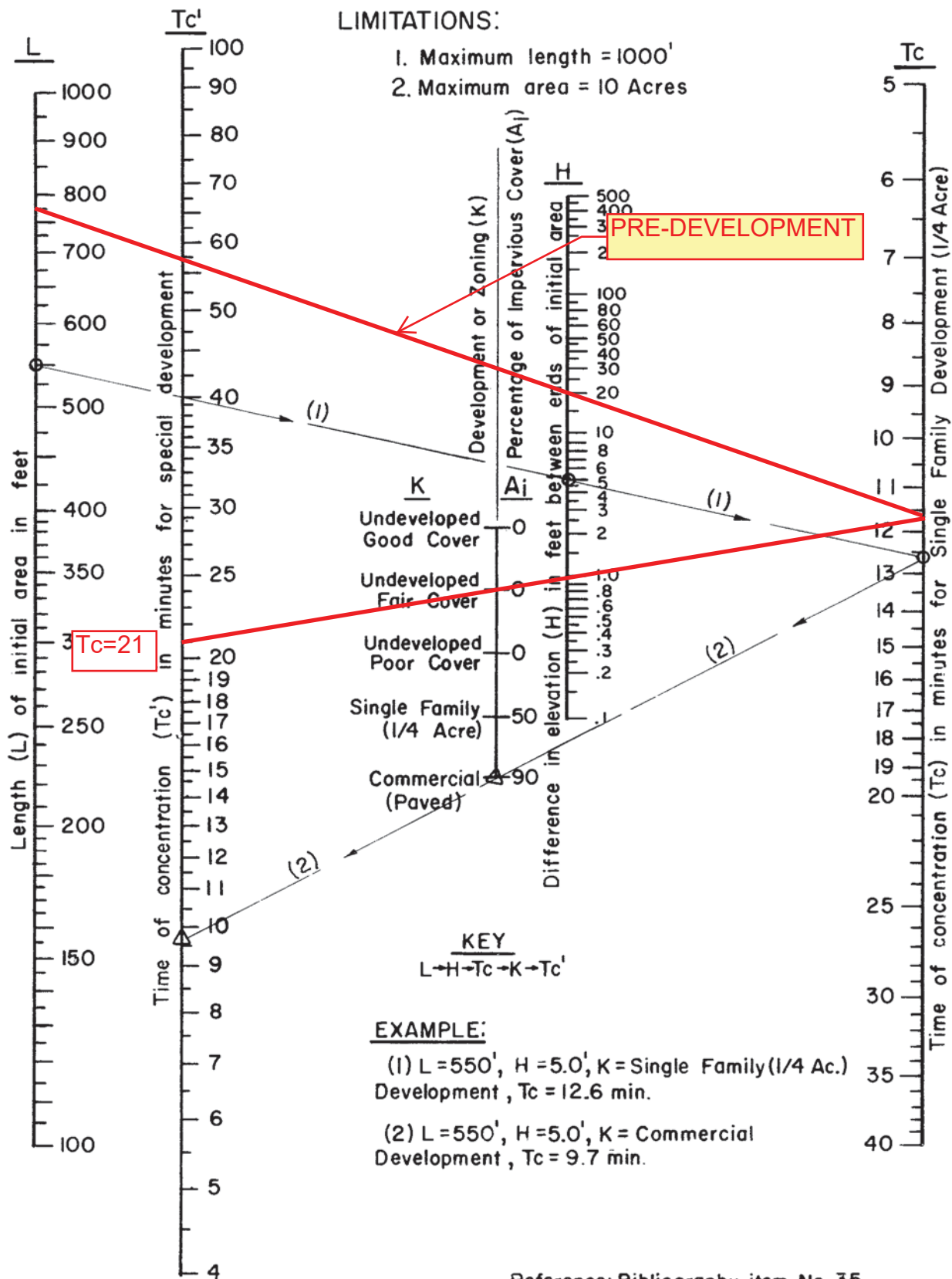
SCALE: 1" = 30'



Reference: Bibliography item No. 35.

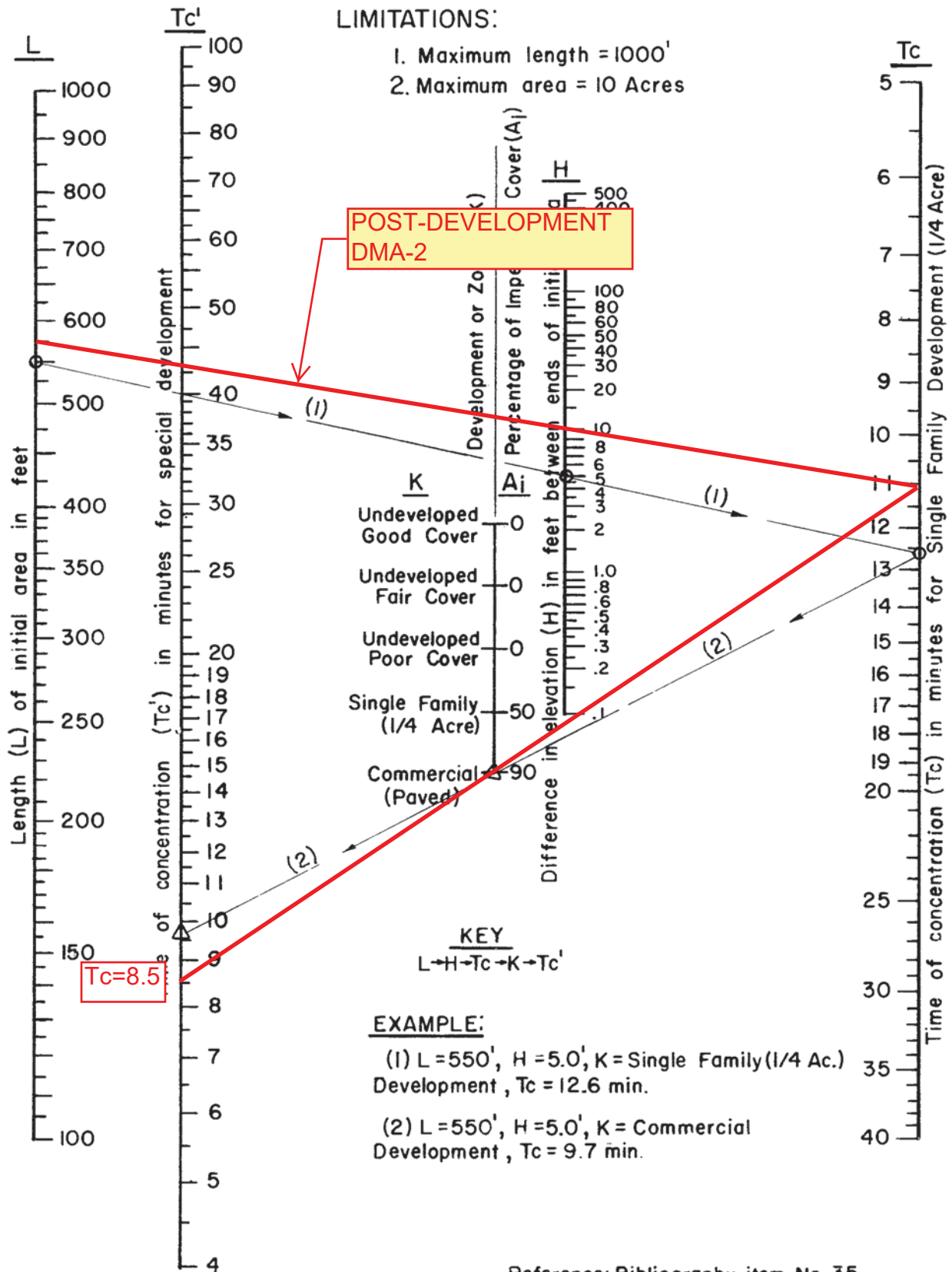
RCFC & WCD
 HYDROLOGY MANUAL

**TIME OF CONCENTRATION
 FOR INITIAL SUBAREA**



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 HYDROLOGY MANUAL

**TIME OF CONCENTRATION
 FOR INITIAL SUBAREA**



Reference: Bibliography item No. 35.

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**TIME OF CONCENTRATION
 FOR INITIAL SUBAREA**

RAINFALL INTENSITY—INCHES PER HOUR

RIVERSIDE			RIVERSIDE (FOOTHILL AREAS)			RUBIDOUX			SAN JACINTO			SUN CITY		
DURATION MINUTES	FREQUENCY		DURATION MINUTES	FREQUENCY		DURATION MINUTES	FREQUENCY		DURATION MINUTES	FREQUENCY		DURATION MINUTES	FREQUENCY	
	10 YEAR	100 YEAR		10 YEAR	100 YEAR		10 YEAR	100 YEAR		10 YEAR	100 YEAR		10 YEAR	100 YEAR
5	2.75	3.92	5	3.14	4.71	5	3.18	4.71	5	2.81	4.16	5	3.25	4.85
6	2.48	3.55	6	2.84	4.26	6	2.87	4.26	6	2.56	3.79	6	2.95	4.40
7	2.28	3.26	7	2.61	3.91	7	2.64	3.91	7	2.37	3.51	7	2.72	4.06
8	2.12	3.03	8	2.42	3.63	8	2.45	3.63	8	2.22	3.29	8	2.53	3.78
9	1.99	2.84	9	2.27	3.41	9	2.30	3.41	9	2.09	3.10	9	2.38	3.55
10	1.88	2.68	10	2.14	3.21	10	2.17	3.21	10	1.98	2.94	10	2.25	3.36
11	1.78	2.54	11	2.03	3.05	11	2.06	3.05	11	1.89	2.80	11	2.14	3.19
12	1.70	2.42	12	1.94	2.91	12	1.96	2.91	12	1.81	2.68	12	2.04	3.05
13	1.62	2.32	13	1.86	2.78	13	1.88	2.78	13	1.74	2.58	13	1.96	2.92
14	1.56	2.23	14	1.78	2.67	14	1.80	2.67	14	1.68	2.48	14	1.88	2.81
15	1.50	2.14	15	1.71	2.57	15	1.74	2.57	15	1.62	2.40	15	1.81	2.71
16	1.45	2.07	16	1.66	2.48	16	1.68	2.48	16	1.57	2.32	16	1.75	2.62
17	1.40	2.00	17	1.60	2.40	17	1.62	2.40	17	1.52	2.25	17	1.70	2.54
18	1.36	1.94	18	1.55	2.33	18	1.57	2.33	18	1.48	2.19	18	1.65	2.46
19	1.32	1.88	19	1.51	2.26	19	1.52	2.26	19	1.44	2.13	19	1.60	2.39
20	1.28	1.83	20	1.46	2.20	20	1.48	2.20	20	1.40	2.08	20	1.56	2.33
22	1.22	1.74	22	1.39	2.08	22	1.41	2.08	22	1.34	1.98	22	1.48	2.21
24	1.16	1.66	24	1.32	1.99	24	1.34	1.99	24	1.28	1.90	24	1.41	2.11
26	1.11	1.58	26	1.27	1.90	26	1.28	1.90	26	1.23	1.82	26	1.36	2.03
28	1.06	1.52	28	1.22	1.82	28	1.23	1.82	28	1.19	1.76	28	1.30	1.95
30	1.02	1.46	30	1.17	1.76	30	1.19	1.76	30	1.15	1.70	30	1.26	1.88
32	.99	1.41	32	1.13	1.70	32	1.14	1.70	32	1.11	1.64	32	1.21	1.81
34	.96	1.37	34	1.09	1.64	34	1.11	1.64	34	1.08	1.59	34	1.18	1.76
36	.93	1.32	36	1.06	1.59	36	1.07	1.59	36	1.05	1.55	36	1.14	1.70
38	.90	1.29	38	1.03	1.54	38	1.04	1.54	38	1.02	1.51	38	1.11	1.66
40	.87	1.25	40	1.00	1.50	40	1.01	1.50	40	.99	1.47	40	1.08	1.61
45	.82	1.17	45	.94	1.41	45	.95	1.41	45	.94	1.39	45	1.01	1.51
50	.77	1.11	50	.88	1.33	50	.90	1.33	50	.89	1.31	50	.96	1.43
55	.73	1.05	55	.84	1.26	55	.85	1.26	55	.85	1.25	55	.91	1.36
60	.70	1.00	60	.80	1.20	60	.81	1.20	60	.81	1.20	60	.87	1.30
65	.67	.96	65	.77	1.15	65	.78	1.15	65	.78	1.15	65	.83	1.25
70	.64	.92	70	.73	1.10	70	.74	1.10	70	.75	1.11	70	.80	1.20
75	.62	.88	75	.71	1.06	75	.72	1.06	75	.72	1.07	75	.77	1.15
80	.60	.85	80	.68	1.02	80	.69	1.02	80	.70	1.04	80	.75	1.12
85	.58	.83	85	.66	.99	85	.67	.99	85	.68	1.01	85	.72	1.08

SLOPE = .550

SLOPE = .550

SLOPE = .550

SLOPE = .500

SLOPE = .530

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HYDROLOGY MANUAL

STANDARD
INTENSITY - DURATION
CURVES DATA

Attachment B

Rational Method Calculations

HYDROLOGY CALCULATIONS

Runoff Flow Calculations

$$Q = CIA$$

Where :

Q = runoff in cubic feet per second (cfs) from a given area.

C= Coefficient of Runoff

I = the time-averaged rainfall intensity (inches/hour) - **1-hr Rainfall**

corresponding to the time of concentration T_c . (See Figures B-3, B-4 and D-1)

(See Hydrology Map for L, H and K factors)

A = Drainage area (acres)

[Soils Maps are at:...\Projects\120374\Hydrology\Plate C-1.42](#)

Return Frequency = **2 years**

PRE-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T_c (min)	$I_{1 \text{ hr}}$ (In/hr)	Q (cfs)
1	5.630	D	0.01	21	0.492	0.03
TOTAL						0.03

Return Frequency = **2 years**

POST-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	$T_{c \text{ (Final)}}$ (min)	$I_{1 \text{ hr}}$ (In/hr)	Q (cfs)
1	2.380	D	0.648	9.5	0.492	0.76
2	3.250	D	0.648	8.5	0.492	1.04
TOTAL						1.79

Return Frequency = **10 years**

PRE-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T_c (min)	$I_{1 \text{ hr}}$ (In/hr)	Q (cfs)
1	5.630	D	0.01	21	0.808	0.05
TOTAL						0.05

HYDROLOGY CALCULATIONS

Return Frequency = **10 years**

POST-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T _c (Final) (min)	I _{1 hr} (ln/hr)	Q (cfs)
1	2.380	D	0.648	9.5	0.808	1.25
2	3.250	D	0.648	8.5	0.808	1.70
TOTAL						2.95

Return Frequency = **25 years**

PRE-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T _c (min)	I _{1 hr} (ln/hr)	Q (cfs)
1	5.630	D	0.01	21	1.01	0.06
TOTAL						0.06

Return Frequency = **25 years**

POST-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T _c (Final) (min)	I _{1 hr} (ln/hr)	Q (cfs)
1	2.380	D	0.648	9.5	1.01	1.56
2	3.250	D	0.648	8.5	1.01	2.13
TOTAL						3.68

HYDROLOGY CALCULATIONS

Runoff Flow Calculations

$$Q = CIA$$

Where :

Q = runoff in cubic feet per second (cfs) from a given area.

C= Coefficient of Runoff

I = the time-averaged rainfall intensity (inches/hour) - **24-hr Rainfall**

corresponding to the time of concentration T_c . (See Plates D-3, D-4.1, D-4.3 and D

(See Hydrology Map for L, H and K factors)

A = Drainage area (acres)

[Soils Maps are at:...\Projects\150647\Hydrology\Plate C-1.42](#)

Return Frequency = **2 years**

PRE-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T_c (min)	I _{24 hr} (In/hr)	Q (cfs)
1	5.630	D	0.01	21	0.098	0.006
TOTAL						0.006

Return Frequency = **2 years**

POST-DEVELOPMENT (on-site)						
Drainage Area	A (acres)	Soils Group	Coefficient C	$T_{c(Final)}$ (min)	I _{24 hr} (In/hr)	Q (cfs)
1	2.380	D	0.648	9.50	0.098	0.151
2	3.250	D	0.648	8.50	0.098	0.206
Total						0.358

HYDROLOGY CALCULATIONS

Return Frequency = **10 years**

PRE-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T_c (min)	I_{24 hr} (ln/hr)	Q (cfs)
1	5.630	D	0.01	21	0.162	0.009
TOTAL						0.009

Return Frequency = **10 years**

POST-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T_c (Final) (min)	I_{24 hr} (ln/hr)	Q (cfs)
1	2.380	D	0.648	9.50	0.162	0.250
2	3.250	D	0.648	8.50	0.162	0.341
TOTAL						0.591

Return Frequency = **25 years**

PRE-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T_c (min)	I_{24 hr} (ln/hr)	Q (cfs)
A	5.630	D	0.01	21	0.201	0.011
TOTAL						0.011

Return Frequency = **25 years**

POST-DEVELOPMENT						
Drainage Area	A (acres)	Soils Group	Coefficient C	T_c (Final) (min)	I_{24 hr} (ln/hr)	Q (cfs)
1	2.380	D	0.648	9.50	0.2001	0.309
2	3.250	D	0.648	8.50	0.2001	0.421
TOTAL						0.730

HYDROLOGY CALCULATIONS

Runoff Volume Calculations

$$V = \frac{Y \cdot A \cdot P_{24}}{12}$$

Where :

V= Volume in acre-ft.

Y = 24-hour storm runoff yield factor for subarea A

$$= \frac{(P_{24} - I_a)^2}{(P_{24} - I_a + S)P_{24}} \quad \text{Formula C.3}$$

P₂₄ = 24-hour storm rainfall from NOAA Precipitation Frequency Server

I_a = initial abstraction

$$= 0.2S \quad \text{Formula C.1}$$

$$S = \frac{1000}{CN} - 10 \quad \text{Formula C.2}$$

A= Drainage area in acres

Return Frequency = 2 Years

PRE-DEVELOPMENT									
Drainage Area	A (acres)	Soils Group	CN AMC II	CN AMC I	S	I _a	P ₂₄	Y	V (Ac-ft)
1	5.630	D	75	57	7.54	1.51	2.35	0.04	0.040
TOTAL									0.040

Return Frequency = 2 Years

POST-DEVELOPMENT									
Drainage Area	A (Acres)	Soils Group	CN AMC II	CN AMC I	S	I _a	P ₂₄	Y	V (Ac-ft)
1	2.380	D	90	78	2.82	0.56	2.35	0.29	0.137
2	3.250	D	90	78	2.82	0.56	2.35	0.29	0.188
TOTAL									0.325

HYDROLOGY CALCULATIONS

Return Frequency = 10 Years

PRE-DEVELOPMENT									
Drainage Area	A (acres)	Soils Group	CN AMC II	CN AMC III	S	I _a	P ₂₄	Y	V (Ac-ft)
1	5.630	D	75	88	1.36	0.27	3.88	0.67	1.23
TOTAL									1.228

Return Frequency = 10 Years

POST-DEVELOPMENT									
Drainage Area	A (acres)	Soils Group	CN AMC II	CN AMC III	S	I _a	P ₂₄	Y	V (Ac-ft)
1	2.380	D	90	96	0.42	0.08	3.88	0.88	0.68
2	3.250	D	90	96	0.42	0.08	3.88	0.88	0.93
TOTAL									1.605

Return Frequency = 100 Years

PRE-DEVELOPMENT									
Drainage Area	A (acres)	Soils Group	CN AMC II	CN AMC III	S	I _a	P ₂₄	Y	V (Ac-ft)
1	5.630	D	75	88	1.36	0.27	6.25	0.78	2.28
TOTAL									2.283

Return Frequency = 100 Years

POST-DEVELOPMENT									
Drainage Area	A (acres)	Soils Group	CN AMC II	CN AMC III	S	I _a	P ₂₄	Y	V (Ac-ft)
1	2.380	D	90	96	0.42	0.08	6.25	0.92	1.15
2	3.250	D	90	96	0.42	0.08	6.25	0.92	1.56
TOTAL									2.710